



1 **Dataset of a 4 km combined seismic and electric streamer survey along**
2 **the embankment of the Po river in Crescentino.**

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12
13 **ABSTRACT**

14 For river embankment characterization non-invasive geophysical investigations are a
15 valuable complementary tool to standard geotechnical testing. To this aim, resistivity and
16 seismic methods are often adopted and can offer complementary information with respect
17 to pore fluid properties and solid skeleton characteristics, respectively. The use of
18 streamer cables for these surveys can reduce the acquisition times, making geophysical
19 investigations ideal for preliminary screening. Few examples of combined seismic and
20 electric streamer systems were already proposed in the literature, but their applicability
21 for long and extensive surveys still need to be demonstrated. Moreover, tailored
22 processing and interpretation methods are necessary given the large amount of data
23 collected with such systems, which can be challenging for standard processing and
24 inversion software.

25 We present a dataset of a 4 km combined seismic and electric streamer survey acquired
26 along the embankment of the Po River in Crescentino, near Torino, in north-west Italy.
27 Data acquisition strategies are discussed and preliminary data analyses are reported. The
28 survey demonstrated the efficiency and potentiality of combined seismic and electric
29 streamer systems in investigating river embankments. All the acquired geophysical data,
30 together with complementary geotechnical and topographic information, are made
31 available in a structured dataset (<https://doi.org/10.5281/zenodo.18183049>). The
32 presented dataset can be an ideal playground to test and benchmark alternative processing
33 and interpretation approaches, and enable an advancement in the state-of-the-art of these
34 characterization methodologies.

35
36 **Keywords:** river embankments, streamer cables, resistivity surveys, seismic surveys,
37 geophysical characterization.

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46 **1 INTRODUCTION**

47 Geotechnical characterization surveys of river embankments, performed with localized
48 geotechnical investigations through drilling or penetration tests, are usually costly and
49 time consuming and provide only local information. Non-invasive geophysical
50 investigations can be complementary to geotechnical testing and provide an efficient
51 characterization of the whole embankment allowing also the number of geotechnical tests
52 to be optimized.

53 Among the available geophysical methods, the ones based on electrical resistivity (ρ) and
54 seismic velocities (mainly shear wave velocity, V_s) have been the most adopted. They
55 offer complementary information with respect to the pore fluid properties (resistivity
56 methods) and the solid skeleton characteristics (seismic methods). Moreover, relevant
57 geotechnical parameters for the evaluation of the stability of embankments and their
58 efficiency (i.e. hydraulic conductivity, porosity, clay content and others) can be
59 potentially obtained through specific correlations applied to these geophysical data
60 combined (e.g. Vagnon et al. 2022a and b).

61 Recently, new acquisition approaches with the use of streamer cables have shown to
62 strongly reduce the acquisition times, making geophysical surveys ideal for a preliminary
63 screening. Seismic surveys can be efficiently carried out with seismic streamers, dragged
64 along the embankments. Numerous successful case studies have been presented during
65 the last three decades (e.g. Van Der Veen et al. 2001; Ivanov et al. 2006; Moura and Senos
66 Matias 2012; Comina et al. 2020a), making this acquisition procedure a well-established
67 methodology. On these data, the multichannel analysis of surface waves (MASW), based
68 on the Rayleigh wave dispersion curve (DC) analysis, is considered the most effective for
69 the determination of V_s profiles (Foti et al. 2018). Several V_s profiles are normally
70 estimated along the embankment to generate a pseudo 2D representation of the V_s field.
71 Several applications of this methodology to embankments, river dykes and earth dams
72 are available in literature (e.g. Lutz et al. 2011; Lane et al. 2008; Min and Kim 2006;
73 Comina et al. 2020a).

74 Conversely, electrical resistivity surveys usually require the insertion of electrodes into
75 the ground, reducing their efficiency and making the acquisition of datasets over long
76 acquisition lines extremely time consuming. Alternative approaches based on Electro
77 Magnetic (EM) methodologies (e.g. frequency or time domain EM) avoid the use of



78 electrodes but rarely allow a comparable detail in the imaged resistivity distribution. To
79 speed up electrical resistivity surveys few examples of electric streamer cables (e.g.
80 Comina et al. 2020b; Umezawa et al. 2022) or combined seismic and electric streamers
81 systems (e.g. Arato et al. 2022) were already proposed in the literature, but their
82 applicability for long and extensive surveys still need to be demonstrated. The main
83 criticalities of these systems is the electric ground coupling that plays an important role
84 in the data quality. Resistivity interpretation usually rely on tomographic inversion of the
85 acquired data (Electric Resistivity Tomography - ERT) that has proved effective to locate
86 fissures and desiccation cracks (e.g. Jones et al. 2014; An et al. 2020), detect animal
87 burrows (e.g. Borgatti et al. 2017), detect seepages and leakage problems (e.g. Al-Fares
88 2014; Busato et al. 2016; Lee et al. 2020), monitor water saturation (e.g. Arosio et al.
89 2017; Tresoldi et al. 2019; Jodry et al. 2019). These approaches were however tested
90 mainly for relatively limited investigation lengths and increasing the survey lengths can
91 pose limitation in most commercial tomographic software.

92 Data acquisition with streamer systems may indeed provide large datasets in short time.
93 The amount of data requires automatic processing techniques to be implemented and
94 parsimonious inversion methods. For DC curves extraction several automatic picking
95 approaches were proposed in the literature and the number of available methods has
96 exponentially raised in recent years (e.g. Zhinong et al. 2021; Colombero et al., 2022; Hui
97 et al. 2024; Pace et al., 2024; Mengyuan et al. 2025; Xingjian et al. 2025; Xiaoping et al.
98 2025). Nevertheless, most of these approaches were verified on synthetic data and on few
99 experimental data, often specifically acquired. With this respect, the availability of an
100 independent dataset, acquired for production purposes, could offer the basis for the
101 validation of the different automatic DC picking algorithms.

102 Most of the developed interpretation approaches of both resistivity and seismic data rely
103 on the evaluation of the geophysical parameters along longitudinal sections of the
104 investigated embankment through specific inverse problem solution. Given the aim of
105 using geophysical data as a fast characterization tool, innovative approaches based on
106 direct data transformations have also been developed, particularly for seismic data (e.g.
107 Comina et al. 2020a; Comina et al. 2022; Lu et al. 2023). Beside the estimation of V_s ,
108 seismic records can also be used to compute energy attributes that allow for detection and
109 location of local anomalies directly (e.g. Nasser-Moghaddam et al. 2005; Zerwer et al.



110 2005; Bergamo and Socco 2014; Colombero et al. 2019). Even though these last methods
111 do not provide physical parameters to be correlated with geotechnical properties, they can
112 be very useful for the identification of the anomalous zones of the embankment to be
113 further assessed through in situ geotechnical testing.

114 Finally, the 3D shape of the embankment can partially affect the geophysical data
115 interpretation and could strongly influence the results of both resistivity and seismic
116 interpretations (e.g. Sjödaahl et al. 2006; Hojat et al. 2019; Karl et al. 2011). Therefore,
117 3D effects should be considered in some specific conditions, even if there is still no
118 standardized approach in their interpretation and in the way these effects should be taken
119 into account. For this reason, more datasets are necessary to test and benchmark different
120 inversion attempts or approximate data transformations.

121 We here present a dataset of a 4 km combined seismic and electric streamer survey along
122 the embankment of the Po River in Crescentino, near Torino, in north-west Italy. Data
123 acquisition strategies with the use of an appropriate seismic and electric streamer cable
124 are discussed and preliminary data analyses are reported. All the acquired geophysical
125 data are described in the paper, organized in a structured dataset and commented in detail
126 in the attached files concerning their format and organization. Complementary
127 geotechnical and topographic information are also included in the dataset as essential data
128 for comparison. The whole dataset is made available
129 (<https://doi.org/10.5281/zenodo.18183049>) for benchmarking alternative processing and
130 interpretation approaches.

131 **2 STUDY SITE, GEOTECHNICAL AND TOPORAPHIC DATA**

132 The investigated test site is the Crescentino embankment of the Po River, near Torino, in
133 north-west Italy (Figure 1). This embankment was built to protect the village of
134 Crescentino and its surroundings, industrial and agricultural activities, from Po river
135 floods. The embankment recently underwent maintenance interventions due to localized
136 seepages, particularly in its terminal eastern portion, nearer to the Crescentino village
137 (Figure 1).

138 Available geotechnical data and on purpose acquired topographic data are hereafter
139 described and included in a specific qgis project (QGIS_topography/Database.qgz). For
140 geotechnical tests, 7 Dynamic Probing Super Heavy (DPSH) tests and two boreholes logs,



141 are included in the project, in numeric format for DPSH and in stratigraphic
142 representation for the borehole logs. For topographic data an on purpose acquired Digital
143 Terrain Model (DTM) and orthomosaic of the embankment are also included.



144
145 Figure 1. Location of the study site in the Crescentino village near Torino, in north-west Italy
146 (inlet), with evidence of the available geotechnical tests and seepage phenomena along the
147 embankment, in red the position of the sections reported in Figure 2 is evidenced (Map data
148 ©2023 Google, acquired from Airbus).

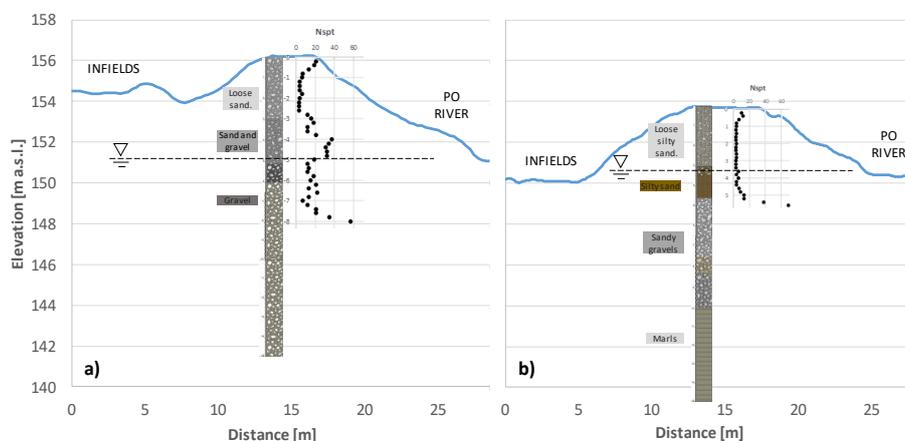
149 2.1 geotechnical data

150 Available borehole logs (Figure 2) over the embankment report a variable subsoil
151 structure whose principal elements are:

- 152 - Embankment body (3 to 6 m thick): constituted of natural material from the
153 surroundings, mainly loose, medium-fine, grey-brown sand with rounded to
154 angular polygenic centimetric gravels and centimetric pebbles; the gravel content
155 in the embankment is higher in the western portion than in the eastern one;
- 156 - Foundation soil (5 to 10 m thick): constituted of rounded, polygenic centimetric
157 gravels and centimeter-sized pebbles in a sandy matrix of medium-coarse,
158 brownish-gray color; localized intercalations of dark gray, slightly compacted
159 silty sand with rounded to angular polygenic centimeter-sized gravels and
160 centimeter-sized pebbles were also observed;
- 161 - Geological substratum (locally evidenced only in the eastern portion of the
162 embankment from 10 m depth): marls ranging from gray-brown to light brown
163 with bluish-gray mottled streaks and abundant centimetric carbonate concretions.



164 In the two available borehole logs the water table is identified at about 5 and 3.5 m depth
165 in the western and eastern parts respectively, showing a slight lowering from west to east,
166 coherently with the Po river flow direction and the attended hydrogeological setting.



167
168 Figure 2. Schematic representation of the shape and geotechnical properties along the a) western
169 and b) eastern portions of the Crescentino embankment, for the location of the sections refer to
170 Figure 1.

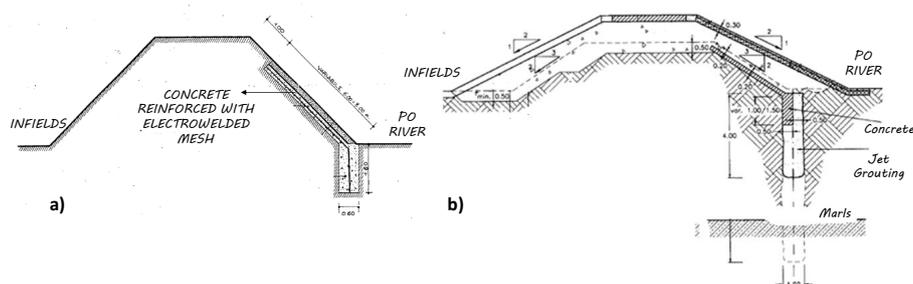
171 Dynamic Probing Super Heavy (DPSH) tests also report variable data along the
172 embankment:

- 173 - A shallow (about 0.5 m) top portion of the embankment, corresponding to the
174 gravelly service road, with medium-high DPSH counts (10 to 15);
- 175 - The embankment body: with low DPSH counts (less than 5), particularly in the
176 eastern portion of the embankment, or locally higher DPSH counts (above 10) in
177 the western portion;
- 178 - The foundation soil with high DPSH counts (generally higher than 20) but with
179 local intercalation of reduced blow counts layers.

180 The embankment has a variable height along its extension. In Figure 2 we show two
181 representative cross-sections located in correspondence of the available borehole logs,
182 and representative DPSH results in the nearby. These sections well represent the
183 variability of construction materials observed along the embankment with the western
184 portion being generally coarser and more compacted (increased gravel content and higher
185 DPSH counts) than the eastern portion, finer and looser (lower gravel content and reduced
186 DPSH counts).



187 The above described lateral variability is also reflected in a different construction design
188 among the two portions of the embankment (Figure 3). In the western portion (Figure 3a)
189 the river side of the embankment was protected in its terminal part with a concrete slab
190 reinforced with an electro welded mesh, and a short (less than 2 m deep) diaphragm at
191 the toe. Conversely, in the eastern portion the actual structure (Figure 3b) is the result of
192 maintenance interventions and rising of the older embankment adopted to prevent
193 seepage. Here, the river side of the embankment was covered with a metal mattress, made
194 by a heavily galvanized net, filled with medium-sized stones. Furthermore, 5 to 6 m deep
195 jet grouting columns were executed at the toe of the older embankment to reach the marl
196 substratum.



197
198 Figure 3. Construction designs adopted along the a) eastern and b) western portions of the
199 Crescentino embankment.

200 Both the adopted design approaches can introduce 3D effects and may cause artefacts in
201 the geophysical data analyses if interpreted in a 2D approach. Therefore, the availability
202 of the above construction details could be very useful for benchmark different elaboration
203 approaches.

204

205 2.2 Topographic data

206 The example sections reported in Figure 2 and the general topographic data along the
207 embankment were obtained from an on-purpose acquired DTM with associated
208 orthomosaic through an Unmanned Aerial Vehicle (UAV) survey. The survey flight was
209 optimized for the long and narrow shape of the embankment by running a single swipe
210 along it, with no lateral overlapping. This approach was attempted to evaluate its
211 effectiveness over similar case studies, in order to reduce the survey time and costs where
212 no transversal extension is needed.



213 A total of 22 photogrammetric targets were homogeneously distributed along the survey
214 path, both on the top of the embankment and at the base of the slopes, serving as Ground
215 Control Points for the georeferencing of the point cloud. The GPS position of the targets
216 were taken with a RTK GNSS antenna (Emlid Reach RS3) and given in absolute
217 coordinates in the UTM WGS84 system, Fuse 32N.

218 The UAV survey consisted in total of 243 images, taken from an elevation of about 90
219 m, with a minimum overlap of 5 to 6 images in the central part. The total covered area by
220 the survey was of about 0.5 square kilometres, with a ground resolution of 3 cm/pixel.
221 The photogrammetric survey was performed with the DJI Mavic Pro 3T drone, controlled
222 with the mission planning software UgCS Pro (SPH Engineering, Latvia). A focal length
223 of 24 mm, an image resolution of 4000x3000 pixels and a pixel size of 1.59 micrometers
224 were adopted. The processing workflow, implemented in Metashape Professional
225 software (Agisoft), was based on the generation of a point cloud from a structure-from-
226 motion algorithm and can be schematized as follows: alignment of all the acquired
227 images; creation and filtering of the sparse point cloud; georeferencing of the point cloud
228 with the photogrammetric targets; creation of the dense point cloud, of the mesh and
229 texture; output creation (orthomosaic and DTM).

230 The accuracy of the photogrammetric model, necessary for the estimation of the overall
231 errors in the generated point cloud, was estimated by selecting some of the targets as
232 Check Points. The average X error was 3.46 mm, the average Y error was 5.6 mm, the
233 average Z error was 2.45 mm.

234 **3 GEOPHYSICAL DATA**

235 Seismic and electrical resistivity measurements were executed along the embankment by
236 means of a combined seismic and electric streamer cable (Figure 4a and b). The streamer
237 integrates seismic receivers and electrodes with a deployment that optimizes resolution,
238 investigation depth and acquisition efficiency. For the seismic part, the streamer is
239 constituted by 48, 4.5 Hz vertical geophones at 1 m spacing. The seismic source is a 40
240 Kg accelerated mass mounted on the survey vehicle (Figure 4c). The first geophone is
241 placed at a 6 m offset from the source. For the electric part, the streamer is similar to what
242 reported in Arato et al. (2022) and is constituted by 13 active electrodes, that can be used
243 both as current and potential electrodes, placed at increasing spacings around their mid-



244 point (Figure 4b). The shorter spaced electrodes are those located around the streamer
245 mid-point (7 electrodes at 2 m separation), while the farthest spaced ones are at the
246 extremities of the streamer (8 m separation). Electrodes are constructed in stainless steel
247 and have the form of brushes (Figure 5a), to increase the contact surface to the ground
248 and reduce coupling resistances. A drip irrigation system completes the electric part of
249 the streamer to further reduce contact resistances at the electrodes. Details of the
250 electrodes, the irrigation system and the resulting effect of their use over the investigated
251 embankment are shown in Figure 5.

252 The resulting streamer total length is of about 50 m. The seismic and electric streamer is
253 dragged by the survey vehicle (Figure 4c) that carries the equipment for seismic and
254 electric resistivity measurements (acquisition systems and water tank). The electrodes and
255 geophones are connected to the acquisition systems (Syscal-Pro, Iris Instruments,
256 georesistivimeter and DaQLink IV, Seismic Source, seismic recording system) by means
257 of multipolar cables. Cables and the irrigation system are collected all together in a plastic
258 coating for easier dragging (Figure 4a).



259

260 Figure 4. Seismic-electric streamer survey: a) picture of the seismo-electric streamer dragged by
261 the survey vehicle during data acquisition, b) scheme of the streamer with evidence of geophones
262 and electrodes disposition and c) picture of the survey vehicle with equipment necessary for
263 seismic and resistivity measurements.



264

265 Figure 5. Seismic-electric streamer survey: a) detail of the adopted electrodes and irrigation
266 system and b) the resulting effect of their use over the investigated embankment.

267 3.1 Survey design and data acquisition

268 The streamer is dragged along the embankment and measurements are carried out at 8 m
269 steps; at each moving step, a single seismic shot and an electric resistivity acquisition
270 sequence are recorded. The survey step was chosen according to simulations as the best
271 compromise between lateral resolution and acquisition efficiency. Specifically, given that
272 the acquisition of electric data is the most time consuming for the surveys, the 8 m value
273 was adopted through numerical simulations of the electrical resistivity surveys.

274 To this aim several forward modelling tests were performed on a simulated 2D model of
275 the embankment, with a background resistivity of 100 Ohm.m and 10 Ohm.m conductive
276 square-shaped anomalies, with a side length of 1m, placed at several depths (every 1 m).

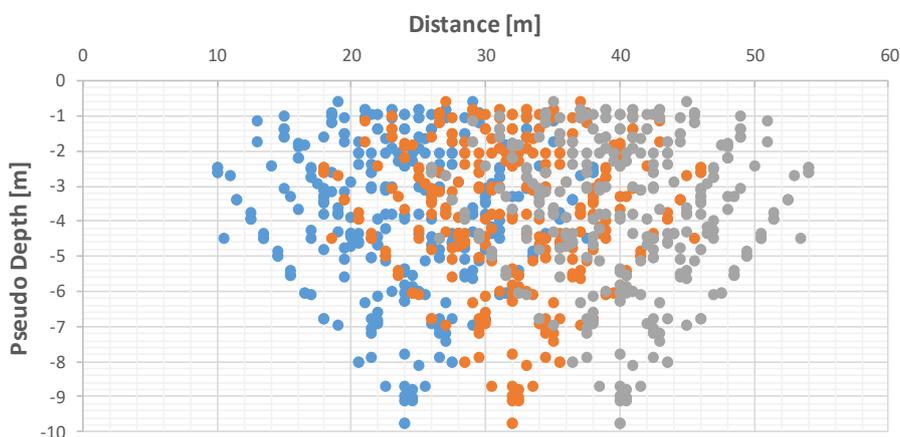
277 These anomalies are intended to represent higher porosity areas potentially leading to
278 dangerous seepages. Different acquisitions were simulated with custom 13-electrodes
279 quadrupole sequences in order to evaluate obtainable resolution and investigation depth.

280 The different quadrupole sequences were designed focusing on speed, therefore including
281 fewer measurements than a standard sequence. Specifically we prioritized: Dipole-Dipole
282 (DD) measurements with adjacent current electrodes, to allow delineate lateral variations;
283 Wenner-Schlumberger (WS) measurements with small current electrodes spacing, to
284 obtain robust measurements near the surface, and because they can be easily linked to DD
285 measurements (using external potential electrodes) and performed simultaneously by the
286 multi-channel instrument; WS measurements with larger current electrodes spacing, for
287 robust deeper measurements.

288 The finally chosen acquisition sequence used both WS and DD quadrupoles and consisted
289 of a total of 318 electric potential readings. In the end, we also checked that the points of



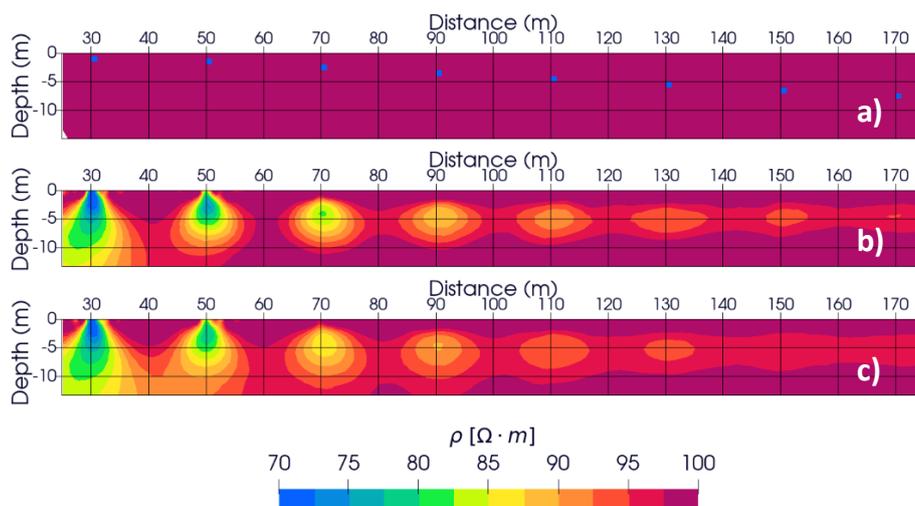
290 the pseudosections of adjacent acquisitions (after moving the streamer each time by the
291 measurement step), were distributed homogeneously. This can be observed in Figure 6
292 for three consecutive acquisition sequences at increasing station positions. It can be noted
293 that a high data density is obtained within the first 5 m of depth, with a very good vertical
294 and lateral resolution, while the data density progressively decreases with depth.



295
296 Figure 6. Example electric acquisition: pseudo depth representation of three consecutive
297 acquisition sequences at increasing station positions.

298 In Figure 7 an example result of the simulations obtained with the finally chosen
299 acquisition sequence and with 4 m and 8 m advancement steps of the streamer system are
300 reported. It can be noticed that no meaningful differences in detecting targets in the first
301 5 meters of depth are observable. The 4 m step acquisition performs only slightly better
302 at greater depths due to increased data coverage of the deeper quadrupoles (see also
303 Figure 6). However, since the 8 m advancement step halves the acquisition time, it was
304 considered a better option, notwithstanding the slight loss in resolution with depth, also
305 considering the attended embankments heights in the survey.

306 From the seismic data point of view, the influence of the acquisition step is instead less
307 problematic. Seismic data can take advantage of the denser geophones distribution along
308 the streamer and even a 8 m advancement step (corresponding to the shot spacing), as the
309 one adopted in the surveys, does not compromise lateral resolution. As an example
310 MASW data elaboration and DC extraction can be performed even at 1 m separation by
311 considering an appropriate geophone moving window and stacking the far offset shots. A
312 preliminary example of seismic data elaboration is reported later in the paper.

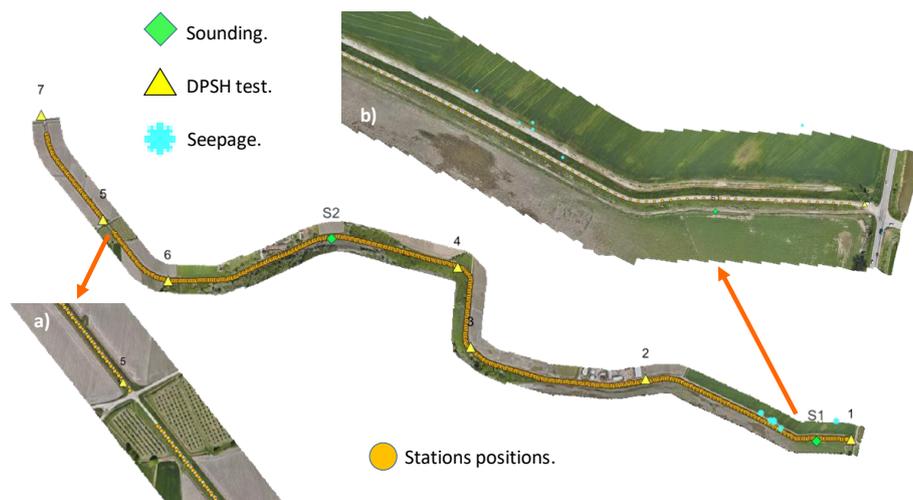


313

314 Figure 7. Numerical simulations of electrical resistivity surveys over several square-shaped
315 conductive anomalies: a) position of anomalies; resulting resistivity models simulating an
316 acquisition with streamer advancement step of 4 m b) and 8 m c).

317 At each station position the location of the electrode nearest to the survey vehicle (which
318 correspond also to the first geophone) was acquired by means of a RTK GNSS antenna
319 (Emlid Reach RS3). In specific positions, particularly in correspondence of embankment
320 curves with a narrower radius, additional GPS data were also acquired along the streamer
321 in its central and terminal parts. These last data, not included in the database, allowed a
322 very reliable positioning of all the electrodes and geophones of the survey (see later). The
323 position data are made available in the qgis project (/QGIS_topography/Stations.gpkg) in
324 absolute coordinates in the UTM WGS84 system, Fuse 32N (Figure 8). Position data are
325 numbered starting from the western part of the embankment, as acquired during survey
326 acquisition.

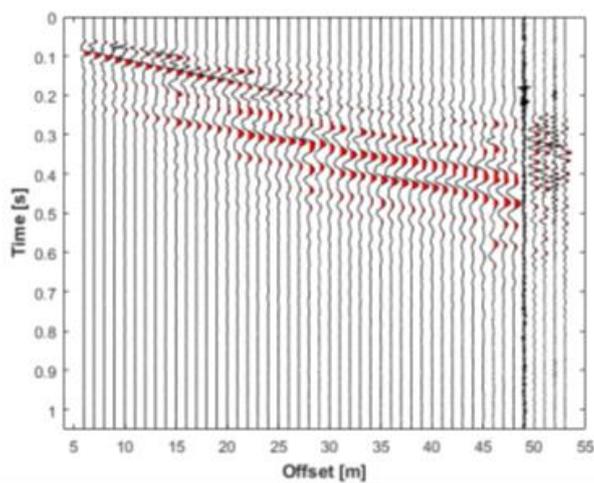
327 During a 7 days' survey campaign, 518 station positions were acquired (Figure 8), with
328 productivity of 800 m/day and about 15 stations per hour of effective acquisition. A
329 relevant amount of time was necessary to deploy and recollect the streamer each day and
330 to refill the water tank for the drip irrigation system. The acquired data provided a
331 continuous coverage of the investigated embankment apart from a small portion of about
332 50 m in the eastern part (Figure 8) related to the crossing of a road which prevented the
333 positioning of the streamer.



334
335 Figure 8. Measured station positions along the Crescentino embankment with details of: a) the
336 small portion of about 50 m in the eastern part with no data and b) the data acquired in the terminal
337 western portion.

338 3.2 Seismic data

339 All the acquired seismic gathers are reported in the database (/Seismic_Survey_Data/) in
340 SEG2 format. Files are ordered using a progressive number from 1, on the western part
341 of the embankment, to 518, in the eastern one. Seismic data were acquired with a 0.5 ms
342 sampling rate, a 0.05 s pretrigger and a 1 s record duration. An example seismic shot is
343 reported in Figure 9.



344
345 Figure 9. Example seismic acquisition: seismic shot with evidence of malfunctioning geophones
346 at the end of the streamer.



347 Data were generally of good quality with just few geophones showing low quality
348 coupling. At some specific locations seismic shots were repeated due to the presence of
349 external disturbances (e.g. tractors and cars). During the first survey days some of the
350 geophones showed malfunctioning (Figure 9 – geophones above number 49). These were
351 moved at the end of the streamer to not compromise the whole seismogram and were later
352 substituted.

353 Starting from the measured GPS positions (Figure 8) the location of each geophone along
354 the streamer was extrapolated using a continuous polyline. The locations of each
355 geophone along the streamer are also included in the database in a specific .csv file
356 (/Seismic_Survey_Data/geophones_positions.csv) ordered as in the acquisition.

357 *3.3 Electric resistivity data*

358 The optimization of quadrupole measurement protocol (for both DD and WS
359 quadrupoles) allowed to perform the 318 electric potential readings sequence with only
360 34 current injections in about one minute, with the addition of few gap-fillers. For each
361 new station position, contact resistances at electrodes were verified; in case of high
362 contact resistances, the drip irrigation was continued till a satisfactory value. This last
363 step increased the acquisition time and occurred generally over the gravellier portions of
364 the road on the embankment.

365 During acquisitions, the current injection time was of 250 ms in each current dipole and
366 the measurements were stacked 3 times; in case of standard deviation greater than 2%,
367 the measurements were stacked 6 times instead.

368 All the acquired electric resistivity data are reported in the database
369 (/Electrical_Tomography_Data/Resistivity_data.csv) in .csv format with indication of the
370 measuring electrodes and the data type of each acquisition. A total of 164724 potential
371 readings were acquired in the 518 executed station positions. Following the same
372 approach previously adopted for the seismic data the location of each electrode along the
373 streamer for each station position was also computed. These latter are also available in
374 the database in a specific .csv file
375 (/Electrical_Tomography_Data/Electrodes_Positions.csv) ordered as in the acquisition.

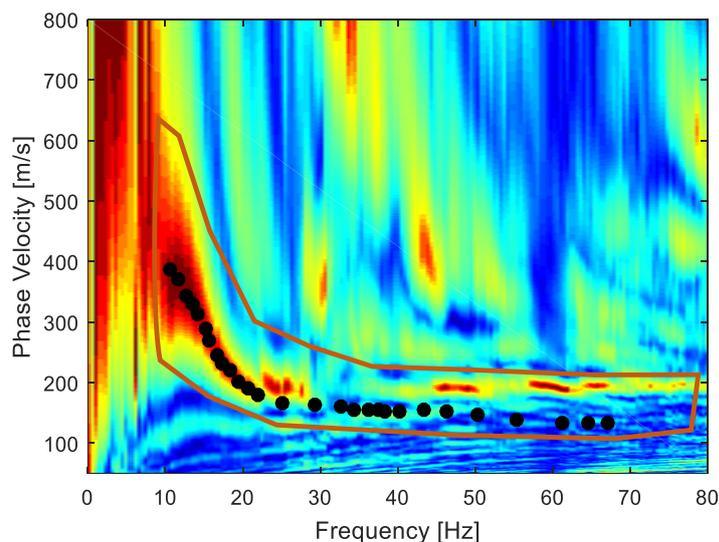


376 4 PRELIMINARY DATA ANALYSES

377 Preliminary analyses were performed on the above described data to provide a first
378 visualization of the Vs and resistivity distributions along the embankment and discuss the
379 potential use of the data also with alternative interpretation approaches. The analyses
380 commented and reported hereafter are not directly included in the database which is
381 focused specifically on the raw data. Authors are however available to share and further
382 compare specific data analyses upon requests.

383 4.1 Seismic data

384 We estimated Rayleigh wave fundamental mode DCs for the whole seismic dataset. An
385 automatic approach for DC extraction, based on the selection of a predefined searching
386 area in the dispersion image, was adopted (Figure 10) and the results were compared with
387 some sample manually picked DCs to check the reliability of the automatic picking. The
388 DCs were picked as the maxima of phase velocity-frequency spectra (Park et al., 1998)
389 computed for a moving window of 24 receivers (the nearest to the shot position) and
390 stacking the spectra from 3 consecutive shots.



391

392 Figure 10. Example of dispersion image from the stacking of 3 consecutive shots. Brown box
393 bounds the adopted search area; black dots corresponds to the manually picked DC.



394 Different combinations of shots and number of geophones were also attempted observing
395 that for an increasing number of geophones the quality of the dispersion image worsen,
396 probably due to a lateral variability along the embankment. The automatic and manually
397 picked DCs are not included in the database but will be available upon request to database
398 users to compare with their DC extraction approaches.

399 Figure 10 shows an example of Dispersion image with the picked DC. As it can be
400 observed higher modes are dominant in the dispersion image in the higher frequency
401 band. This effect is quite common over embankment structures due to the presence of the
402 compacted road above it or to 3D effects linked with the geometry of the embankment.
403 The presence of these higher modes can affect the automatic DC extraction.

404 A reliable analysis of the DC was possible in the 10 to 60 Hz frequency band for most of
405 the shots. Given the velocity ranges of the investigated materials (see Figure 10) this
406 frequency band correspond to maximum and minimum wavelengths of about 30-40 m
407 and 2 m respectively. Given these ranges it can be therefore considered that the
408 investigation depth of the seismic survey is slightly higher than the one of resistivity
409 surveys and with a comparable resolution at shallower depths.

410 *4.2 Electric resistivity data*

411 Electric resistivity data were analysed to remove anomalous apparent resistivity values.
412 Data having negative resistivities, outliers (data with very high or very low resistivity,
413 and isolated) and few data having higher standard deviation were removed. This result in
414 about 10 % of the acquired measurements to be rejected.

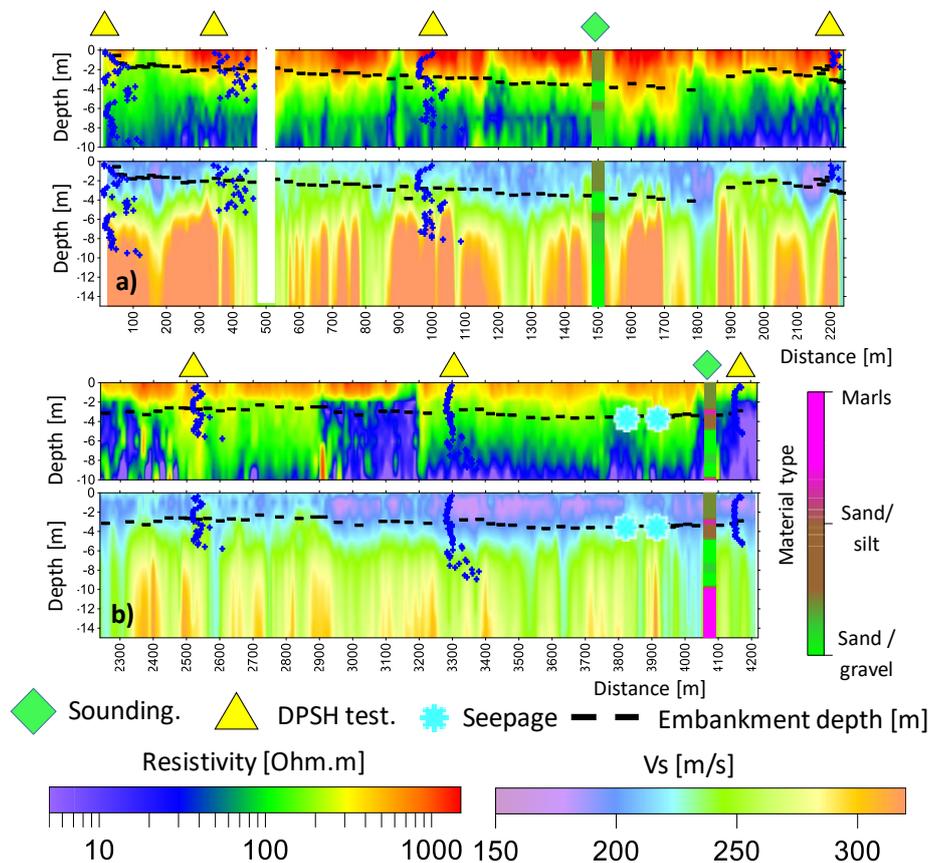
415 Preliminary 2D and 3D inversions were also attempted on a small portion of the electric
416 resistivity dataset by means of the Resipy software (Blanchy et al. 2020). For 3D
417 inversion the shape of the embankment and the exact location of the electrodes was
418 properly considered from the available topographic data.

419 **5 DATA VISUALIZATION AND DISCUSSION**

420 Figure 11 shows the filtered apparent resistivity data presented in the form of a pseudo
421 section and the DCs represented in the form of 1D time average Vs models. The time
422 average Vs models were estimated by directly transform the DCs through a linear
423 wavelength-depth transformation (W-D transform) proposed by Comina et al. (2022) on
424 the basis of the analysis of a wide flat-file database of surface waves dispersion curves



425 and Vs profiles (Passeri et al., 2021). Both pseudo sections were obtained by gridding the
 426 experimental data with a kriging interpolation in a grid of 1 m vertical spacing and 10 m
 427 horizontal spacing. The available geotechnical and borehole information along the
 428 embankment is superimposed to the geophysical data and the location of known seepage
 429 events is also reported. Also, the estimated embankment depth is visualized over the other
 430 data.



432 Figure 11. Visualization of the resistivity and time average Vs data along the western a) and eastern
 433 b) portions of the embankment together with available geotechnical information and evidence
 434 from localized seepages.

435 From these preliminary data visualization, it can be evidenced that the lateral variability
 436 along the embankment evidenced by the geotechnical investigations is correctly depicted
 437 by geophysical data and that there is a good correspondence with the information from
 438 geotechnical investigations.



439 Along the western part (Figure 11a) the embankment is generally characterized by high
440 resistivity values (above 800 Ohm·m) and medium-high velocity values (generally higher
441 than 200 m/s), indicating the presence of mainly medium compacted sands inside it.
442 Below the embankment a decrease in resistivity related to the presence of the water table
443 is observed, and an increase in velocity due to the gravellier nature of the subsoil.
444 Specifically, the marked increase in velocity (above 300 m/s) well correspond with the
445 evidence from DPSH tests.

446 Conversely, along the eastern part (Figure 11b), the embankment is generally
447 characterized by medium resistivity values (300 to 800 Ohm·m) and generally lower
448 velocity values (lower than 200 m/s) indicating the presence of mainly medium loose silty
449 sands inside it. Below the embankment, a decrease in resistivity similar to the western
450 part is observed. However, a less sharp increase in velocity is noted which may be related
451 to the lateral transition from gravels to compacted marls (substratum).

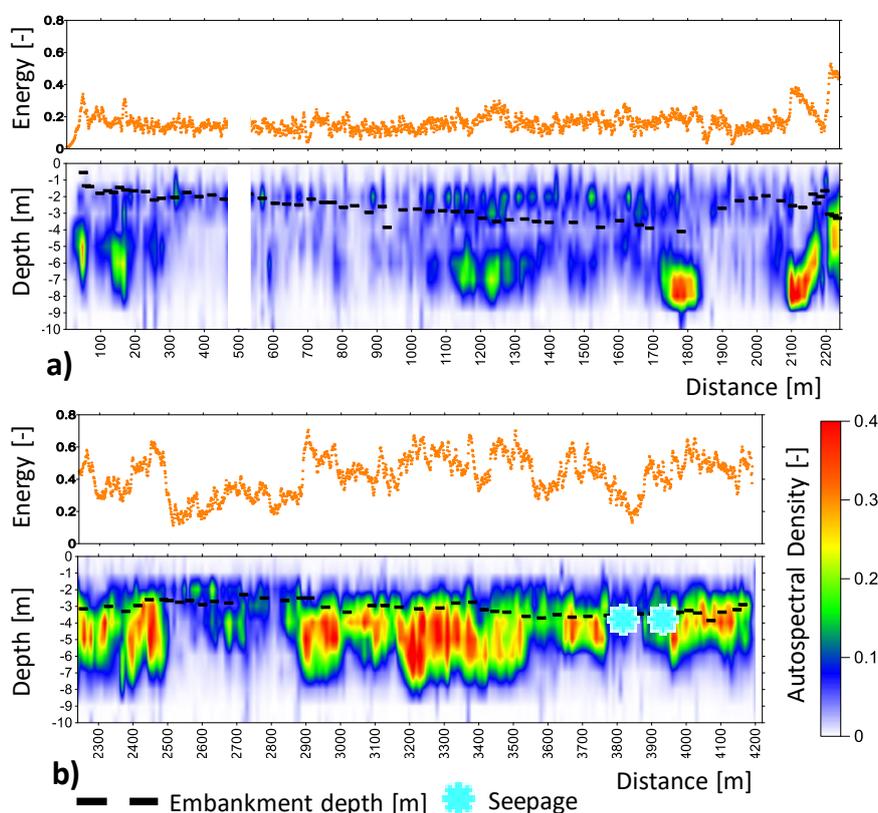
452 Some localized low resistivity and low seismic velocity areas emerge from the surveys.
453 These areas could be potentially related to critical portions within the embankment and
454 shallow foundation soil, characterized by increased water content (low resistivity data)
455 and reduced compactness (low seismic velocity).

456 The data visualization proposed in Figure 11 offers a fast and simple way to picture the
457 embankment conditions. This can be beneficial in situations in which a fast preliminary
458 evaluation of the state of health of an embankment is required (e.g. in the case of surveys
459 conducted near, or in foresee of, significant rain and/or flood events). In these conditions,
460 the proposed data acquisition and visualization approach can allow an almost immediate
461 evaluation of geophysical parameters distribution at the site and a direct imaging of the
462 resistivity and seismic properties, even while the acquisition is in progress, during the
463 dragging of the streamer along the embankment.

464 To better pinpoint localized anomalies, the analysis of seismic data was also
465 complemented with the computation of the Energy (Ei) and Autospectral Density (ADi)
466 of all the acquired seismic traces along the profile. These data attributes showed both in
467 numerical simulations and in real case histories their ability in detecting and locating
468 anomalies having lower acoustic impedance than the surrounding material (Colombero et
469 al. 2019). These type of anomalies is usually observed with an increase in both Ei and
470 ADi and could be related to potential weakness zones along the embankment. In Figure



471 12 we display these attributes along the embankment. For the ADi the same wavelength
472 to depth conversion used for the DC was adopted to transform the frequency in an
473 estimated depth. To allow better understanding of the lateral variations, both the
474 parameters were normalized along the whole embankment length.

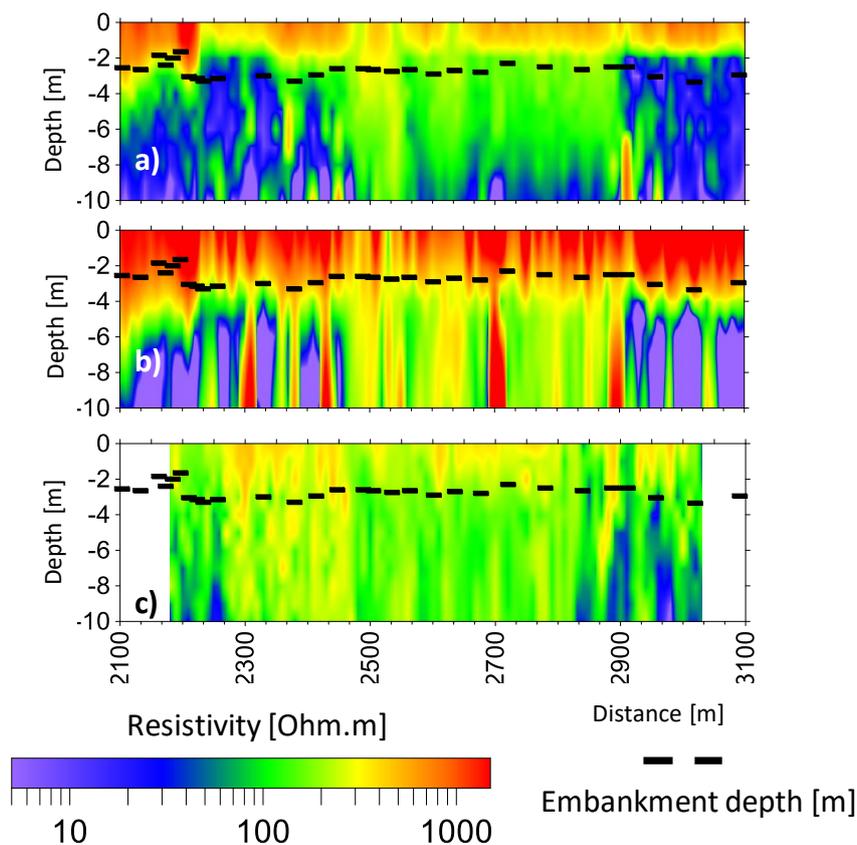


475
476 Figure 12. Normalized Energy (top) and Autospectral Density (bottom) of all the acquired seismic
477 traces along the western a) and eastern b) portions of the embankment together the evidence from
478 localized seepages.

479 The different properties of the western and eastern parts of the embankment are confirmed
480 also by this analysis. Indeed, along the western part (Figure 12a) low E_i values and few
481 localized high ADi spots are evidenced, indicating a generally uniform condition and few
482 anomalies. Conversely along the eastern part (Figure 12b) several high E_i peaks and high
483 ADi spots are evidenced. Particularly from the progressive 2900 m on localized seepages
484 are well known within the embankment and could be the cause for the anomalies in the
485 seismic parameters.



486 Notwithstanding the good correspondence observed between geophysical data and
487 geotechnical information, some of the evidenced anomalies need to be further
488 investigated since they may be related to 3D effects near embankment curves or specific
489 embankment design features. In Figure 13 the results of the 2D and 3D inversions of
490 resistivity data in a 1 km portion of the embankment are reported and compared to the
491 measured pseudosection. The investigated portion is located between progressives 2100
492 and 3100 in correspondence to a significant embankment curve (i.e. at the position of
493 DPSH 3 in Figure 8).



494
495 Figure 13. Resistivity data inversion over a small portion of the embankment between
496 progressives 2100 and 3100 m in correspondence to a significant embankment curve (i.e. DPSH
497 3 in Figure 8): comparison between a) pseudo section b) 2D and c) 3D inversions.

498 From this analysis it can be evidenced that even if the general resistivity distribution is
499 quite similar between pseudosection, 2D and 3D inversions some differences related to
500 the different processing approaches emerge. Specifically, 2D inversions tend to provide



501 a result with sharper resistivity contrasts, while the 3D inversion provides a smoother
502 image reducing the resistivity contrasts below the embankment. Therefore, as already
503 evidenced in literature (e.g. Sjödaahl et al. 2006; Hojat et al. 2019) when the aim of the
504 survey is the definition of potential correlation between resistivity data and geotechnical
505 parameters, the use of 3D inversion is to be considered mandatory for an appropriate
506 quantification.

507 Similar and more detailed data analyses than the one here presented could be object of
508 further studies on both electric and seismic datasets. For example, with respect to seismic
509 data also the compressional wave velocity distribution could be investigated with first
510 break picking tomographic approaches or with devoted data transform approaches from
511 the same DCs (e.g. Comina et al., 2020a). For electric data, 3D inversions with the
512 inclusion of a-priori information related to the embankment design and structure (Figure
513 3) could be also attempted for a better understanding of lateral effects.

514 **6 CONCLUSION**

515 The use of combined seismic and electric streamer surveys for the characterization and
516 screening of embankments also for relevant investigation lengths was shown in this work.
517 The executed surveys allowed to cover the whole investigated embankment in relatively
518 short acquisition times, providing information on the distribution of the geophysical
519 parameters. A comprehensive dataset of the acquired electrical resistivity and seismic
520 data was built, including also independent geotechnical and topographic information. The
521 dataset is made available for benchmarking alternative processing and interpretation
522 approaches in order to contribute to the advancement of the state of the art of these
523 characterization methodologies. Further data processing and inversion are foreseen in the
524 near future for a more complete characterization together with combined resistivity and
525 seismic analyses with the aim of specific geotechnical parameters estimation.



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535 program (PNRR, Missione 4 “Istruzione e ricerca” – Componente 2 “dalla ricerca all’impresa”,
536 Investimento 1.3, finanziato dall’Unione europea – Nextgeneration EU, pubblicato dal
537 Politecnico di Milano con Decreto del Direttore Generale D.D. Rep. n. 7881/2024, Prot. n.
538 161661/2024 del 02/07/2024).

539 **Data Availability**

540 All the data described in the paper are available in a structured dataset at: Vergnano, A., Comina,
541 C., Socco, L. V., Chieppa, D., & Arato, A. (2026). Database of a 4-km seismic and electric
542 streamer survey: the embankment of the Po River near Crescentino, Piedmont, Italy. [Data set].
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569 river embankments by means of coupled remote sensing and geophysical techniques: Lessons
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