

1 **StageIV-IRC: A High-resolution Dataset of Extreme Orographic Quantitative**
2 **Precipitation Estimates (QPE) Constrained to Water Budget Closure for**
3 **Historical Floods in the Appalachian Mountains**

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12 **Abstract**

13 Quantitative Flood Estimation (QFE) in complex terrain remains a grand challenge in
14 operational hydrology due to the lack of accurate high-resolution Quantitative Precipitation
15 Estimates (QPE) for operational forecasting and for calibrating hydrologic models. Here, we
16 present a high-resolution (i.e., 250m, 5-minute-hourly) QPE dataset for 215 extreme rainfall events
17 occurred in 26 gauged mountainous basins in the Appalachian Mountains from 2008 to 2024. This
18 dataset is developed by applying inverse rainfall corrections (IRC) derived from physically-based
19 rainfall-runoff modeling (Liao and Barros, 2022 and 2023) to the Next Generation Weather Radar
20 (NEXRAD) Stage IV analysis (4 km resolution, hourly). The corrected Stage IV analysis QPE is
21 referred to as StageIV-IRC (StageIV with Inverse Rainfall Correction). The unique advantage of
22 this StageIV-IRC QPE dataset is its agreement with ground-based rainfall measurements while
23 achieving water budget closure at the storm-flood event scale and minimizing uncertainty from
24 initial conditions using the Initial Condition Correction (ICC) module. ~~within observational~~
25 ~~uncertainty of streamflow observations, which is the gold standard in hydrological modeling.~~ This
26 dataset is the first QPE dataset aiming to improve QFE in the complex terrain by reducing biases
27 for extreme precipitation events, and it can be used to evaluate the skill of hydrologic models in
28 the same basins and support model calibration. The StageIV-IRC QPE dataset is publicly available
29 at <https://doi.org/10.5281/zenodo.14028866>, and improved initial soil moisture maps for the
30 studied extreme precipitation events, derived from the ICC module in the same IRC framework,
31 are available in the same repository (Liao and Barros, ~~2025e~~2025c).

32

33 **1. Introduction**

34 Over the past few decades, extreme precipitation has become an increasingly important
35 research topic due to its social, economic, and environmental impacts (e.g., Alimonti et al., 2022;
36 Wernberg et al., 2013). Studies show that both total annual precipitation and extreme precipitation
37 events have increased in the US and in other parts of the world during the last century (e.g., Milly
38 et al., 2002), often resulting in floods (e.g., Pielke and Doughton, 2002), and flash floods in the
39 context of complex terrain due to steep slopes (e.g., Schumacher, 2017; Czigány et al., 2010).
40 Flash floods are characterized by fast rainfall-runoff responses on the scale of a few hours (< 6
41 hours) after extreme precipitation events for watershed areas often ranging from a few tens to
42 hundreds of square kilometers (e.g., Borga et al., 2014; Lumbroso and Gaume, 2012). As one of
43 the deadliest natural hazards, flash floods are often associated with landslide events (e.g., Tao and
44 Barros, 2014; Gupta et al., 2016; Deijns et al., 2022) and cause loss of life and property damage
45 (Špitalar et al., 2014), such as recently in the last three years in the Appalachian Mountains, USA,
46 and in Southern Spain. Despite extensive studies to improve flash flood simulations in small
47 headwater basins, hydrological skill scores (e.g., Kling-Gupta Efficiency or KGE) remain poor at
48 event scales largely due to significant difficulties involved in estimating highly localized
49 orographic precipitation in complex terrain, which in turn implies that hydrologic models are not
50 calibrated using forcing representative of realistic extreme events (e.g., Andrieu et al. 1997;
51 Huffman et al., 2007; Mtibaa and Asano, 2022).

52 Current approaches involved in precipitation measurement and Quantitative Precipitation
53 Estimation (i.e., QPE) broadly include in-situ point-scale observations using rain gauges and
54 disdrometers, and remote spatial observations using ground-based radar and space-based sensors.
55 In complex terrain, there is often a scarcity of in situ measurements due to difficult access. For

56 example, the rain gauge network from NASA's Integrated Precipitation and Hydrology Experiment
57 is the only relatively dense rain gauge network installed at high elevations in the entire
58 Appalachians (e.g., Barros et al. 2014). Other QPE products (e.g., radar QPE data) are plagued by
59 uncertainties from various sources (e.g., ground clutter artifacts, retrieval uncertainties, and radar
60 viewing geometry (Villarini and Krajewski, 2010; Arulraj and Barros, 2021; Kreklow et al., 2020;
61 Huffman et al., 2007; Andrieu et al., 1997; Durden et al., 1998). Numerical weather prediction
62 (NWP) is an alternative to measurement. However, QPE products from NWP models are
63 characterized by significant uncertainties when evaluated against rain gauges (e.g., Zhang and
64 Anagnostou, 2019), leading to large flood simulation errors when used as inputs to hydrological
65 models, or introducing large structural uncertainty when used for model calibration (e.g., Tao et
66 al., 2016; Weiland et al., 2015; Diomede et al., 2008; Kobold and Suselj, 2005). Due to these
67 uncertainties and errors involved, focus has been directed towards enhancing QPE using various
68 methods: data merging of raingauge and radar precipitation (e.g., McKee and Binns, 2016;
69 Goudenhoofdt and Delobbe, 2009; Delrieu et al., 2014; Nanding et al., 2015; Sideris et al. 2013;
70 Schiemann et al. 2011), combined radar reflectivity and retrieval corrections (e.g., Vignal et al.,
71 2000; Shao et al., 2021; Dinku et al., 2002), and data assimilation into NWP models (e.g.,
72 Rafiecinasab et al., 2015; Wehbe et al., 2020). Rain gauge and disdrometer measurements are often
73 used as references for these QPE optimization approaches (e.g., Harrison et al., 2000; Shao et al.,
74 2021; Fulton et al., 1998). The 'ground truth', however, has its own error (e.g., spatial
75 representativeness, wind artifacts around the gauge orifice, and calibration, among others;
76 Kochendorfer et al., 2017), and fails to capture highly localized orographic enhancement (e.g., Prat
77 and Barros, 2010b; Gentilucci et al., 2021; Buytaert et al., 2006). Gauge-radar fusion often relies
78 on geostatistical assumptions that are primarily distance-based (e.g., Areerachakul et al., 2022;

79 Cassiraga et al., 2021; Wang et al., 2020; Maggioni and Massari, 2018), lacking the full picture of
80 complex basin topography, which has a regulating role in orographic precipitation processes.

81 To address this long-standing QPE challenge in complex terrain, a general QPE error
82 quantification framework was developed leveraging widely available quality United States
83 Geological Survey (USGS) streamflow observations at the outlet of headwater basins in complex
84 terrain, consisting of 2 distinct paths: 1) rain gauge bias correction, and 2) grid-level QPE
85 correction constrained to watershed-scale water budget closure. The first pathway includes rain
86 gauge bias corrections at gauge locations both at the diurnal and climate scales, and the
87 geostatistical distribution of rain gauge biases across a basin. The second pathway includes an
88 innovative inverse QPE correction method by backward propagating runoff uncertainty using a
89 hydrological model via streamlines to precipitation at storm-event scale, and the methodology is
90 termed Inverse Rainfall Correction (IRC), which is developed by the same authors (Liao and
91 Barros, 2022 or LB22). The IRC was initially developed in the Southern Appalachians and later
92 extended to the headwater basins over a span of 2,000 km from south to north along the entire
93 Appalachian Mountains for all headwater basins. It is worth noting that rain gauges are only
94 available in the Southern Appalachians, thus elsewhere QPE corrections the StageIV product was
95 downscaled to 250-m first and then submitted to the IRC without bias corrections or any other
96 intermediate corrections as in LB22. in other headwater basins only went through the process of
97 IRC without any prior rain gauge bias corrections. The generalizability of the IRC framework,
98 regardless of rain gauge bias corrections beforehand, is demonstrated in is overall successful as
99 documented in Liao and Barros (2023). (see Figure12).

100 LB22 found that initial soil moisture uncertainty causes inferior performance of IRC
101 because large initial condition errors lead to significant uncertainties in travel time distributions.

102 Soil moisture is considered a particularly important factor among soil properties due to its
103 significant role in affecting the generation of runoff, hence dramatically altering the timing of flood
104 front and its magnitudes (e.g., Vivoni et al., 2007; Marchi et al., 2010; Penna et al., 2011), and soil
105 moisture can vary dramatically at hourly timescales, changing from fully saturation levels to
106 wilting point levels conditional on the specific texture and other properties of the soils (Grillakis
107 et al., 2016). Initial soil moisture conditions can therefore determine whether a rainstorm produces
108 a major flash flood or not (e.g., Komma et al., 2007; Zehe and Blöschl, 2004). However, due to
109 the limited availability of soil moisture sensors, there are not many studies quantifying the impact
110 of soil moisture on runoff simulation (e.g., Silvestro et al., 2019; Laiolo et al., 2016; Zappa et al.,
111 2011; Uber et al., 2018). Liao and Barros (2025b) developed an Initial Condition Correction (ICC),
112 which is based on travel time distributions and is coupled with the general IRC approach,
113 demonstrating large improvements in initial soil moisture estimation. Note that when
114 implementing the IRC and ICC, we are using a fully distributed physics-based uncalibrated model
115 (i.e. Duke Coupled Hydrological Model, DCHM) that has been used successfully for more than
116 two decades for hydrologic studies in the Southern and Central Appalachians (e.g., Tao and Barros,
117 2013, 2014, 2018 and 2019; Tao et al. 2016; Yildiz and Barros 2004, 2007 and 2009), and
118 consequently uncertainty from model structure and model parameters is assumed to be small.
119 Hydrological model parameters certainly have an impact on rainfall-runoff response, but they are
120 generally only of secondary importance compared to the precipitation proper and antecedent soil
121 moisture distributions, especially for smaller basins (e.g., Dobler et al., 2012; Mockler et al., 2016).

122 In this work, IRC and ICC are combined into one ~~framework . structure~~ (referred to as the
123 IRC-ICC framework ~~in Liao and Barros (2025b).~~) to construct an improved QPE dataset aiming
124 to close the water budget at the scale of storm-flood events along the entire Appalachian Mountains

125 [ranges](#), (e.g., [Liao and Barros 2022 and 2025b](#)). The study region is set to be the Appalachian
126 Mountains because they are prone to extreme precipitation and flash floods due to orographic lift
127 of moisture-laden air masses coming from the Gulf of Mexico and the Atlantic Ocean (e.g., Troch
128 et al., 1994; Smith et al., 2011; Liao and Barros, 2023). A recent example is Hurricane Helene,
129 which caused over 200 deaths and over \$50 billion in property damage in the Southeast US in
130 September 2024. The IRC-ICC framework is employed in 26 headwater basins and 215 extreme
131 events (during 2008-2024) using the Next Generation Weather Radar (NEXRAD) StageIV dataset
132 as original inputs, at a spatial and temporal resolution of 250 m and 5 minutes, respectively, and
133 the improved post IRC-ICC QPE data (i.e., StageIV-IRC) are made available in this study.

134 The manuscript is organized as follows. The data sources and the QPE error quantification
135 framework, which consists of rain gauge bias correction and the IRC-ICC framework, are detailed
136 in Section 2. Section 3 presents this new dataset (StageIV-IRC) along with data assessment from
137 various aspects. Section 4 discusses the potential application of this new dataset and future work.
138 Section 5 provides access to the dataset and a summary of the work.

139

140 **2. Data and Methodology**

141 **2.1 Radar QPE StageIV**

142 The NCEP/EMC StageIV is a precipitation estimation product, developed using hourly and
143 6-hourly radar-rain gauge precipitation analyses at regional scales (Lin and Mitchell, 2005). In
144 complex terrain, it is known that radar QPE suffers from the blockage of topography, overshooting
145 and retrieval uncertainties, leading to large uncertainties in rainfall estimation. In 2007, as part of
146 the ground validation (GV) of the Precipitation Measurement Missions (PMM) program by NASA

147 (e.g., Prat and Barros, 2010a and 2010b), 34 tipping bucket raingauges were installed in the
148 Southern Appalachians and have been well-maintained since 2007 (e.g., Barros et al., 2014). In
149 this work, rain gauge measurements from a GV rain gauge network ~~in the Southern Appalachians~~
150 are utilized to reduce StageIV uncertainties in the Southern Appalachians.

151

152 2.2 GV Rain Gauge Observations

153 A rain gauge network in support of PMM GV was installed in the Pigeon River basin for
154 the 10 year 2007-2018 period (Barros et al. 2014). A map of this rain gauge network is plotted in
155 Figure 1. Every rain gauge is labelled with a number, and exact locations are documented in Table
156 1. This rain gauge network is regularly visited and maintained at least three times a year, including
157 on-site cleaning and calibration. In this study, these rainfall measurements are used as a basis to
158 adjust hourly StageIV QPE. Note these rain gauge measurements can be downloaded at
159 <http://dx.doi.org/10.5067/GPMGV/IPHEX/GAUGES/DATA301> (Barros et al., 2017). Besides
160 rain gauges, a network of Parsivel disdrometers was installed during 2013-2014, with each
161 disdrometer location denoted by the letter P in Figure 1. These disdrometer data were only used
162 for independent evaluation because of short records. It is worth noting that rain gauges are installed
163 mostly along the ridges while disdrometers are generally located at lower elevations.

164

165 <Figure 1 here please>

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167 **2.3 Methodology**

168 The methodology of this work includes ~~four major elements~~~~three major components~~: a) rain gauge bias and climatology corrections where raingauge data are available, b) downscaling of radar precipitation, c) grid-scale QPE correction by closing the water budget using stream gauge measurements, and d) basin and event selection procedures and model setup.

172 **2.3.1 Rain Gauge Bias-Corrections**

173 A schematic drawing of the rain gauge ~~bias~~-correction framework to derive gauge-improved QPE (named StageIV_{DBKC}) is provided in Figure 2. ~~The~~ ~~where the~~ ~~subscripts~~ DBKC ~~referes to~~ 'Downscaled', 'Bias correction using rain gauge measurements at gauge locations', 'Correction from Kriging interpolation in 2D', and 'Climatological corrections', respectively.

177 <Figure 2 here please>

178

179 First, to make meaningful comparison between StageIV estimates and rain gauge measurements spatially ~~s in spaae~~, a fractal downscaling algorithm is used to create StageIV_D at 1km from the original StageIV at 4-km resolution. Subsequently, bias correction using raingauge measurements is employed to create StageIV_{DB} at hourly timescales. StageIV_{DB} data are then evaluated against the rain gauge climatology from 2008 to 2017 to reduce biases that depend on weather regime, and climatological biases are then spatially interpolated using the ordinary Kriging method. The resulting dataset is named StageIV_{DBKC} (abbreviated as STIV_{DBKC}).

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187 2.3.2 Fractal downscaling

188 The methodology for fractal downscaling was first proposed by Bindlish and Barros (1996)
189 and subsequently demonstrated through various applications to precipitation downscaling from
190 models (Bindlish and Barros, 2000) and remote sensing data (Nogueira and Barros, 2015; Tao and
191 Barros, 2010). Here, a brief description is presented.

192 The assumption of self-similarity is imposed in fractal downscaling approach. The
193 parameters used in this approach involve: fractal dimension D , Hurst coefficient H , and the spectral
194 exponent β that are related through the following equations:

$$195 \quad D = \frac{7-\beta}{2} \quad (1)$$

$$196 \quad H = \frac{\beta-1}{2} \quad (2)$$

197 The parameter β describes rainfall statistics across different spatial scales, and it is
198 calculated as the slope of the power spectral density curve in the 2D Fourier domain of the rainfall
199 field (log-log plot). The parameter H is the Hurst coefficient which is a measure of autocorrelation
200 strength with higher value representing stronger autocorrelation. The ~~power spectral density of a~~
201 ~~2D field in Fourier domain~~ 2D Fourier transform of a rainfall field $z(x, y)$ is calculated as the
202 following:

$$203 \quad Z(u, v) = \left(\frac{L}{N}\right)^2 \sum_{x=0}^{N-1} \sum_{y=0}^{N-1} z(x, y) \exp\left[-\frac{2\pi i}{N}(ux + vy)\right] \quad (3)$$

204 where N is the total number of grid points of the rainfall field $z(x, y)$ with grid size being
205 the L , u and v correspond to frequency indices in the Fourier domain in each direction. [Using Eq.](#)
206 [3.4](#) the averaged power spectral density is given:

207
$$S_j = \frac{1}{L^2 N_j} \sum_1^{N_j} |Z(u, v)|^2 \quad (4)$$

208 where N_j denotes the number of points that meet the following condition: $j < \sqrt{u^2 + v^2} <$
 209 $j + 1$. ~~The mean power spectral density and the wavenumber k (Eq.5) are related by a power law (Eq. 6) and the mean power spectral density, and k is defined as below:~~

210 ~~There is roughly a power-law relationship between~~
 211 ~~the wavenumber k (Eq.5) are related by a power law (Eq. 6) and the mean power spectral density,~~
 212 ~~and k is defined as below:~~

$$k = \frac{2\pi}{\sqrt{u^2 + v^2}} \quad (5)$$

213
$$S \sim k^{-\beta-1} \quad (6)$$

214 ~~By applying a logarithmic transformation, the power-law relation between S and k is~~
 215 ~~linearized, and Specifically, the corresponding S value when wavenumber $k = 1$ is referred to as~~
 216 the roughness factor, which is a representation of the variance of the field.

217 Assuming rainfall fields have self-similar statistics ~~from coarse resolution to fine resolution~~
 218 ~~expressed by a power-law~~, then fine scale rainfall fields can be generated ~~from the coarse scale~~
 219 ~~radar observations~~ by preserving these self-similar statistics. This is accomplished by creating a
 220 Brownian surface at desired fine scale resolution while sharing the same spectral slope and
 221 roughness factor as the original rainfall field based on Bindlish and Barros (1996):

222
$$Z_D(u, v) = \frac{Z_b(u, v)}{k_r^{(\beta - \beta_b)/2}} \exp \left[\frac{1}{2} \left(S_{r,1} - \frac{\beta + 1}{\beta_b + 1} S_{r,2} \right) \right] \quad (7)$$

223 where β , β_b , $Z_D(u, v)$ and $Z_b(u, v)$ are the spectral slope of 2D original rainfall field, the
 224 spectral slope of the Brownian surface, interpolation surface in the Fourier domain and original
 225 Brownian surface, respectively; k_r is the wavenumber and $S_{r,1}$ and $S_{r,2}$ are the roughness factors
 226 of the 2D original rainfall fields and Brownian surface. Due to the non-uniqueness of Brownian

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227 surfaces, multiple replicates of interpolation surfaces Z_D ~~must~~ ~~can~~ be ~~generated~~~~obtained~~. In this
228 study, an ensemble of ND (~~Number of Downscaled samples~~) interpolation surfaces is derived ~~from~~
229 ~~the original StageIV product where ND=50 following Nogueira and Barros (2015), and~~ thus ~~fifty~~
230 ~~ND_~~rainfall fields ~~realizations~~ at finer resolution preserving the same rainfall statistics at coarse
231 resolution ~~is~~ ~~are~~ generated, and the ensemble mean was calculated. ~~In this work, a total of ND=50~~
232 ~~ensemble downscaled rainfall fields are created similar to Nogueira and Barros (2015), and Finally,~~
233 ~~a series of the~~ rainfall bias-correction steps ~~is~~ described in Figure 2 ~~are applied and is applied~~ to
234 the ensemble mean of the downscaled rainfall fields.

235

236 2.3.3 ~~Climatology~~ Bias Corrections

237 The *first* phase of bias correction is carried out at the event scale: a linear regression is
238 established between rain gauge measurements and collocated downscaled radar pixel estimates
239 using the following formula:

$$240 R_g^t(i_g, j_g) = \kappa R_r^t(i_g, j_g) + \varepsilon \quad (8)$$

241 where R_r and R_g represent radar and rain gauge measurements respectively, κ and ε are the
242 slope and the intercept of a polynomial fit between R_r and R_g . Hourly StageIV_D estimates and
243 corresponding rain gauge observations in the same StageIV_D pixel were identified if at least 2 rain
244 gauges in the same StageIV_D pixel measure non-zero rainfall. A linear regression was applied to
245 all StageIV_D pixels within one standard deviation of the regression line at an hourly timescale by
246 assuming homogeneity of variances or homoscedasticity.

247 The *second* phase of bias correction is done at decadal scale: aiming to reduce systematic
248 radar errors caused by retrieval uncertainties and viewing geometry in complex terrain,

249 demonstrating strong diurnal (time of day) and seasonal (weather regime) error dependencies due
250 to ~~missed~~ detection of shallow rainfall systems related to radar overshooting in the Southern
251 Appalachian when comparing against 10-year rain gauge observations (e.g., [Prat and Barros,](#)
252 [2010b](#); Wilson and Barros, 2014; [Duan et al. 2015](#); ~~Arulraj~~ and Barros, 2017). For this purpose,
253 when rain gauge observations are < 2 mm/hr and Stage IV_D estimates are 0 mm/hr, the StageIV_D
254 value was automatically replaced by the rain gauge observations, which is referred to as the Light
255 Rainfall Correction (LRC). Moreover, if StageIV_D ~~rainfall intensity is zero estimates equal to 0~~
256 where at least one collocated rain gauge observation is > 2 mm/hr, then StageIV_D estimates are
257 replaced by the mean of all collocated rain gauge observations, namely Mean Rainfall Correction
258 (MRC). Lastly, for highly localized precipitation (i.e., ~~fewer less~~ than 2 raingauges register
259 nonzero rain in the study domain) which is normally ~~associated produced by with small-scale~~
260 convective activity, the rainfall differences between the StageIV_D and the local rain gauge
261 observations were bilinearly distributed across nearby ~~25~~ grids (a 5x5 grid square centered at the
262 StageIV_D pixel) – Convective Rainfall Correction (CRC). For most of the raining hours, there are
263 more than 2 rain gauges ~~with nonzero recorded rainfall~~, in which case the differences ~~at each~~
264 ~~pixel~~ between radar estimates and raingauge measurements were spatially interpolated using
265 ~~ordinary Kriging a geostatistical interpolation method (e.g., ordinary Kriging)~~, which is referred to
266 as the Global Rainfall Correction (GRC).

267

268 2.3.4 Ordinary Kriging

269 Ordinary Kriging is a geostatistical interpolation method that generates artificial values of
270 a variable at a specific location, aiming to minimize spatial variance. In this work, rainfall
271 differences between raingauge observations and StageIV_{DB} are calculated and distributed across

272 the entire basin using a spatial variance model, which is commonly referred to as a semi-variogram
 273 model. Specifically, a spherical semi-variogram model is used. Literature regarding the choice of
 274 semi-variogram models and their properties can be found (e.g., Li and Heap, 2008; Oliver and
 275 Webster, 2015; Zimmerman and Zimmerman, 1991). Bohling (2005) pointed out that spherical
 276 models reach the maximum variance for relatively shorter spatial lags, therefore more suitable to
 277 capture highly nonlinear and localized orographic precipitation (McBratney and Webster, 1986):

$$278 \quad \gamma(h) = C_0 + (C - C_0) \left(\frac{3h}{2d} - \frac{1}{2} \left(\frac{h}{d} \right)^3 \right) \quad \text{if } 0 \leq h \leq d \quad (9.1)$$

$$279 \quad = C \quad \text{if } h > d \quad (9.2)$$

$$280 \quad \gamma_{0i} = \frac{1}{N_A} \sum_{k=1}^{N_A} \gamma_{ki} \quad (9.3)$$

$$281 \quad \gamma_{00} = \frac{1}{N_A} \sum_{k=1}^{N_A} \sum_{l=1}^{N_A} \gamma_{kl} \quad (9.4)$$

282 where h is the lag, d is the range, C and C_0 are the sill and nugget values of the semi-variogram
 283 model, N_A is the number of raingauges. The nugget is assumed to be zero if local variability and
 284 measurement error are neglected at the point scales (Diggle and Ribeiro, 2007). The interpolated
 285 rainfall difference at a location x_0 $Z_{ok}^*(x_0)$ is calculated using a weighted combination of all
 286 available differences at gauge locations $G(x_i)$ multiplied by Ordinary Kriging weights λ_i^{ok} :

$$287 \quad Z_{ok}^*(x_0) = \sum_{i=1}^n \lambda_i^{ok} G(x_i) \quad (10.1)$$

$$288 \quad \sum_{i=1}^n \lambda_i^{ok} = 1 \quad (10.2)$$

289 Optimal Kriging weights can be obtained by a series of linear equations using the Lagrange
 290 multiplier μ method:

$$\begin{pmatrix} \gamma_{11} & \dots & \gamma_{n1} & 1 \\ \vdots & \ddots & \vdots & \vdots \\ \gamma_{1n} & \dots & \gamma_{nn} & 1 \\ 1 & \dots & 1 & 0 \end{pmatrix} \begin{pmatrix} \lambda_1^{OK} \\ \vdots \\ \lambda_n^{OK} \\ \mu \end{pmatrix} = \begin{pmatrix} \gamma_{01} \\ \vdots \\ \gamma_{0n} \\ 1 \end{pmatrix} \quad (11)$$

292 In this work, Ordinary Kriging interpolates differences between radar data and raingauge
 293 observations to produce gauge-corrected STIV_{DBKC} dataset. [An example sequence of rainfall fields](#)
 294 [to illustrate the step-wise corrections described procedures mentioned in Sections 2.3.1-2.3.4 is](#)
 295 [shown in Figure A1.](#)
 296

297 2.3.5 Precipitation Assessment Metrics

298 Assessment metrics include the following: bias and root mean square error between radar
 299 estimation and raingauge measurement, false alarm rate, the probability of detection ([PD](#)), threat
 300 score ([TS](#)) and Heidlke skill score ([HSS](#)), following McBride and Ebert, 2000. An instance when
 301 both radar QPE and rain gauge observation exceed a specified rain rate threshold is a hit (H); when
 302 observation matches the criterion and radar QPE does not, it is classified as a miss (M); if the
 303 opposite happens, then it is a false alarm (FA). The calculation of these metrics relied on a
 304 collection of Hs, Ms, and FAs:

$$\text{Bias} = \frac{1}{N} \sum_{n=1}^N (O_n - R_n) \quad (12)$$

$$\text{RMSE} = \sqrt{\frac{1}{N} \sum_{n=1}^N (O_n - R_n)^2} \quad (13)$$

$$\text{FR} = \frac{\text{FA}}{\text{H} + \text{FA}}, 0 \leq \text{FR} \leq 1 \quad (14)$$

$$\text{PD} = \frac{\text{H}}{\text{H} + \text{M}}, 0 \leq \text{PD} \leq 1 \quad (15)$$

309
$$TS = \frac{H}{H+FA+M}, 0 \leq TS \leq 1 \quad (16)$$

310
$$HSS = 2 * \frac{Z * H - FA * M}{((H+FA) * (Z+FA)) + ((M+H) * (M+Z))}, -1 \leq HSS \leq 1 \quad (17)$$

311 where O is the rain gauge observation, R is the radar QPE, and N is the number of points. Z
312 represent the number of zeros, meaning both raingauge and radar do not register a rainfall record
313 above a predefined threshold. A threat score (TS) of 0.5 means over 50% of cases meet the
314 criterion, and the higher the better. An HSS of 0 means a forecast has the same performance as a
315 random guess.

316

317 **2.3.6 Inverse Hydrologic Correction**

318 At flash flood timescales in headwater basins, streamflow uncertainty and precipitation
319 uncertainty are strongly connected in a nonlinear way through rainfall runoff processes. Liao and
320 Barros (2022) developed a Lagrangian-based framework named Inverse Rainfall Correction (IRC),
321 allowing backpropagating streamflow uncertainty to precipitation inputs in space and time through
322 an uncalibrated distributed hydrological model (i.e., DCHM), achieving water budget closure at
323 the event scale in small headwater basins. As stated earlier, the uncertainties associated with
324 parameters and the hydrological model DCHM are neglected since the model configurations have
325 been used and improved over the past two decades for this region accounting for various soil,
326 vegetation, and river processes (e.g., Tao and Barros, 2013, 2014, 2018 and 2019; Yildiz and
327 Barros, 2005 and 2007; Lowman and Barros, 2016), and the IRC framework has been tested in

328 multiple headwater basins extensively in this region with consistent success. The detailed
329 description of the IRC is provided in Section 2.3.8 and Appendix A.

330 It is worth noting that IRC is a general framework to improve QPE at the watershed scale
331 that can be incorporated into any distributed hydrological models. Liao and Barros (2025a, 2025b)
332 investigated the impact of model structure uncertainty and initial condition uncertainty on IRC and
333 then the downstream product: the resulting IRC_-improved QPE. The results suggest with
334 improved watershed physics at finer resolution (e.g., river bank storage, Liao and Barros, 2025a),
335 river routing algorithms (e.g., XY routing, Liao and Barros, 2025a) and improved antecedent soil
336 moisture distributions (Liao and Barros, 2025b), post-IRC QPE demonstrate realistic precipitation
337 features at high resolution that are aligned with basin topography with ridges associated with
338 higher precipitation than valleys in general, showing a significant improvement from the original
339 StageIV dataset which is characterized by unnatural boxy precipitation patterns in complex terrain
340 due to resolution issues and over or underestimation depending on topography and distance from
341 the radar site.

342 As briefly mentioned before, LB22 reviewed various sources of uncertainty that can
343 prevent post-IRC QPE from achieving water budget closure, among which initial condition
344 uncertainty in soil moisture is a noteworthy source. Improved initial condition estimation results
345 in significantly improved post-IRC precipitation features in complex terrain by better capturing
346 transient travel time distributions (Liao and Barros, 2025b). They found that the uncertainty tied
347 to initial conditions is more significant for less extreme events. Nevertheless, the initial condition
348 correction method is coupled with the IRC framework, and the complete framework is named the
349 IRC-ICC framework. The specifics regarding the IRC, ICC, and IRC-ICC are schematically drawn
350 in Figure 3.

351

352 <Figure 3 here please>

353

354 Using the definitions of characteristic timings shown in panels c) and d), characteristic flow
355 regime windows are identified. In principle, the number and the size of the windows depend on
356 the complexity of the hydrograph. ICC is only applied to windows 2 and 5 in this example, which
357 represents a segment of the hydrograph characterized by the differences between rising points in
358 observations and simulations, and a segment characterized by slow recession, respectively. The
359 assumption is that precipitation uncertainty regulates streamflow differences during peak flows
360 (i.e. windows 3 and 4). W_{nm} represents the framework state after window m for iteration n . The
361 resolution settings for the DCHM are: spatial resolution: 250m, and temporal resolution: 5 minutes.

362 **2.3.7 Implementation of Lagrangian Tracking**

363 A flood event is simulated by the DCHM at the basin outlet with grid-based time-varying
364 velocity fields for different soil layers. When the precipitation starts (i.e. basin-averaged
365 precipitation $> 0.1\text{mm/hr}$), new particles (passive tracers) are launched at the same frequency of
366 model temporal resolution (5 minutes), but only at non-zero precipitation grids in all soil layers
367 following the velocity fields calculated by the DCHM, and the tracking resolution is 10 seconds,
368 amounting to a release of approximately 600,000 particles for basin with an area of 120km^2 over
369 a 24-hour period. During the tracking phase, each particle is saved along with information
370 regarding its source location (grid-point where it originates), time of release t_i , and travel time t_T
371 (t_T is defined as the difference between current time t and the time of release t_i , i.e., $t_T = t - t_i$).
372 Multiple particles from different source locations can have the same travel time, which is the basis

373 for identifying the number of trajectories contributing to the hydrograph at the outlet as a function
374 of time.

375

376 **2.3.8 QPE Correction Using IRC**

377 At time t , the water difference $wd(t)$ between the observed and simulated streamflow over
378 the time Δt between two consecutive discharge observations represents the fraction of runoff that
379 eventually leaves the basin as streamflow. Errors in precipitation forcing propagate to the runoff,
380 under the assumption of negligible model and parameter uncertainties, $wd(t)$ can be entirely
381 attributed to precipitation error, which is the focus of this work.

$$382 \quad wd(t) = [Q_{obs}(t) - Q_{simu}(t)] \times \Delta t \quad (18)$$

383 The subscripts *obs* and *simu* refer to observed and simulated discharge, respectively.
384 The strategy for the inverse rainfall correction (IRC) using hydrograph analysis is to follow the
385 trajectories available from the Lagrangian tracking backward from the basin outlet to the source
386 locations at time t_i and apply a correction at the source locations proportional to the original QPE
387 magnitude to reduce wd at time t . Detailed formulas with a conceptual drawing can be found in
388 Appendix A. The embedded assumption is that larger QPE values have larger uncertainties. Note
389 that QPE corrections that happened earlier in time will have an impact on runoff simulation at
390 future times, and this is the reason why the IRC framework is a recursive framework. The detailed
391 rainfall correction steps can be found in (Liao and Barros, 2022).

392

393 **2.3.9 Methods for Reducing Uncertainties from Other Sources**

394 As briefly mentioned before, uncertainties from other sources (e.g., model physics, model
395 numerical formulation, antecedent soil moisture conditions, etc.) impact travel time distributions
396 and simulated streamflow to a higher or lesser degree depending on location, antecedent
397 conditions, and storm system. Previous studies demonstrate that, for flood-producing events in
398 small headwater basins, streamflow response is largely controlled by precipitation inputs (e.g.,
399 Iwasaki et al., 2020). In this section, we briefly describe the methods used to minimize the impacts
400 from other sources to enhance water budget closure using the IRC approach.

401 As discussed in the Introduction DCHM has been used in the Appalachian Mountains at
402 event-scale (e.g., Tao and Barros, 2013, 2014, 2018 and 2019; Tao et al. 2016) and at seasonal and
403 interannual scales (Yildiz and Barros 2005, 2007 and 2009), and thus extensive analysis of
404 parameter uncertainty and model structure uncertainty has been conducted previously. Recent
405 improvements to the flood routing algorithm have resulted in significant improvements in flood
406 peak timing in headwater basins to reconcile the hydraulics of flood wave propagation on steep
407 slopes at the highest elevations with milder slopes at intermediate elevations in the valleys (Liao
408 and Barros, 2025a). Their results also suggest meandering effects, riverbank storage, and initial
409 soil moisture distributions can impact the early rising period of the hydrographs. Significant and
410 consistent improvements are made when introducing an initial condition correction (ICC) module
411 to reduce initial condition uncertainty (Liao and Barros, 2025b). This innovative ICC module is
412 coupled with the IRC framework. The red arrows in Figure 3e indicate where ICC is executed in
413 the general architecture of the IRC framework, and the specifics of the ICC module are described
414 below.

415 Particles launched during the IRC process that reached the outlet at time t are traced back
416 directly to the IC timing or time 0, and their locations at the IC timing are shown in the bottom
417 maps in Figure 3d (referring to control points of time t). The downstream area of the control points
418 has shorter transportation time to arrive at the outlet (e.g., water difference ΔS_1), and the upstream
419 area of the control points takes longer to get to the basin outlet (e.g., water difference ΔS_2).
420 Similarly, soil moisture in the impacted area can greatly impact the size of ΔS_2 and flow conditions
421 after the timing t_2 . Assuming initial conditions are only impactful during the early period and late
422 recession of the hydrograph, which is supported by the fact that these events are flood-producing
423 events with large QPE uncertainties dominating the vicinity of peak flow, ICC is used for
424 hydrological windows outside the peak flow windows. Following the same notation (backward-
425 in-time) in the IRC framework (Eq. 18), $wd(t)$ is calculated as the flow volume difference
426 between observed and simulated streamflows for the time interval defined by t and $t - \Delta t$. A
427 ‘band’ of region can therefore be identified, that is, a region formed by control points of time t
428 and control points of time $t - \Delta t$. This ‘band’ is then referred to as the impacted area of initial soil
429 moisture for time t , meaning basin discharge between time $t - \Delta t$ and time t is impacted by initial
430 soil moisture at the delineated impacted area. Finally, $wd(t)$ is then converted to soil moisture
431 content and added to initial soil moisture within the impacted area (i.e. the ‘band’) and the details
432 can be found in Liao and Barros (2025b).

433

434 2.3.10 Hydrological Skill Metrics

435 The Kling-Gupta Efficiency (KGE) is calculated using observed and simulated streamflow
436 statistics at observation resolution τ (here 15 minutes) in this work:

437
$$KGE_{\tau} = 1 - \sqrt{(r - 1)^2 + \left(\frac{\sigma_{sim}}{\sigma_{obs}} - 1\right)^2 + \left(\frac{\mu_{sim}}{\mu_{obs}} - 1\right)^2}$$
 (19)

438 where r is the correlation between simulations and observations, σ_{obs} is the standard
 439 deviation of observed discharge, σ_{sim} is the simulated discharge standard deviation, μ_{sim} and
 440 μ_{obs} represent the average simulated and observed streamflow values, respectively.

441 The relative volume error (EV) is the relative difference between simulated flood volume
 442 and observed flood volume:

443
$$EV = \frac{V_{sim} - V_{obs}}{V_{obs}}$$
 (20)

444 Where V stands for volume of the flood. An $EV > 0$, and an $EV < 0$ mean overestimation and
 445 underestimation, respectively.

446 EPT refers to the error in peak flow timing between observations and simulations. For its
 447 calculation, only the highest peak is selected for calculating EPT if more than one peak is present.
 448 In this work, EPT is determined by considering the entire flood rising limb to account for the
 449 steepness of the rising limb, specifically, both the flood starting timing and the maximum flood
 450 timing from the flood front rising limb are used for calculating the EPT.

451 EPV or error in peak volume (Q_{max} , cubic meters per second) is a relative error calculated
 452 using peak flows from observations and simulations, and the equation is below:

453
$$EPV = \frac{Q_{max_{sim}} - Q_{max_{obs}}}{Q_{max_{obs}}}$$
 (21)

454

455 **2.3.11 Study Domain and Model Setup**

456 ~~Twenty-eight~~An initial 30 gauged headwater basins with areas and quality streamflow
457 records were are identified selected from south to north along in the Appalachian Mountains
458 (Figure 4, Table 3). s as illustrated in Figure 1, with basin drainage area ranging from 50 km² to
459 500 km². It is demonstrated in Figure 4 that these basins scatter across the entire Appalachians.
460 For example, Basin01 and Basin30 are over 2,000 km apart, with diverse weather and climate
461 regimes, and large differences in geomorphology and hydrogeology. The smallest basin (Basin 07)
462 and the largest basin (Basin 12) were discarded because their areas were less than 20 km² and
463 greater than 600 km², respectively due to being too small for IRC at 250 m resolution and too
464 large for IRC due to complex hydrologic response from different catchments not all impacted by
465 the same rainfall event, significantly different than other basins. Note that an additional 2 basins
466 (Basin 13 and 14) were later discarded from inclusion in the final data set as explained later in
467 Section 3.2 and Section 3.2.1. Therefore, making the final published StageIV-IRC product
468 includes 26 basins and 215 events in 26 basins.

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469
470 <Figure 4 here please>

471

472 Soil-related parameters wereare downloaded from a global high-resolution (1 km) soil data
473 repository (Zhang et al., 2018). For each basin, the vertical hydraulic conductivity remains the
474 same for the entire soil column. The lateral hydraulic conductivity in the unsaturated zone was
475 assumed to be two to three orders of magnitude larger than the vertical conductivity in the shallow
476 soil layers, with higher values where the stone fraction in the soils is higher (Carlson, 2010; Freeze

477 and Cherry, 1979). The final scaling factors were obtained through simple sensitivity analysis to
478 match the curvature and slope of the observed subsurface runoff recession curves (e.g., Linsley et
479 al., 1982; Chen and Kumar, 2001; Yildiz and Barros, 2007), and scaling factors are finally
480 determined as: 1500, 150, 15 and 1.5 for layer 1 (0-10 cm below terrain surface), layer 2 (10-75
481 cm below terrain surface), layer 3 (75-200 cm below terrain surface) and layer 4 (2-20 m below
482 terrain surface), respectively. No parameter optimization is done in this work, as the primary focus
483 of this work is to develop a QPE dataset that can consistently close the water budget while
484 controlling uncertainties from other sources, largely advancing the understanding of QPE
485 uncertainties across climate, weather, and geomorphological regimes.

486 Flood-producing events ~~were have been~~ selected for ~~the 28~~²⁸ ~~twenty eight~~ headwater basins
487 ~~with areas ranging between 50-500 km²-~~ (Table 3) for recent years from January 2021 to April
488 2024. A qualified event is determined based on the observed peak flow, which must surpass 95%
489 of available flow measurements for each basin. The choice of 95% is a compromise because 99%
490 would yield too few events, while 90% would be too close to the annual flood. Additionally,
491 rainfall runoff response time must be shorter than or equal to 6 hours to be qualified as a flash
492 flood event. Only warm season precipitation events from 2021 to 2024 are finally considered.
493 Here, the warm season is specifically defined as from April 1st to September 30th. Note: data
494 quality control is enforced, and events with missing streamflow records are discarded.

495 For the Cataloochee Creek Basin (Basin05), located in the SAM known to have
496 experienced multiple flash floods in the past (Tao and Barros, 2013 and 2014), Liao and Barros
497 (2023) created a Historical Flood Record database (HFR) that includes a large number of extreme
498 rainfall events from 2008 to 2017. The event selection criteria when developing HFR also use the
499 same 95% flow threshold method. The difference is that the HFR also includes multiple winter-

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500 time liquid precipitation events that result in cold-season flash floods. In total, there are 54 warm-
501 season events for Basin 05 in HFR, and these events are also used to expand the study sample size
502 in this work.

503 To ~~initialize~~~~warm-up~~ the DCHM, a traditional spin-up approach is used with iterative runs
504 for the hydrological year of 2021 (from the end of April to the end of September), and it generally
505 reaches equilibrium after 3-5 iterations. Subsequently, DCHM is continuously running from the
506 beginning of October 2021 onwards, to derive initial conditions for events after September 30th,
507 2021. During this spin-up process, no parameter calibration is involved. The initial conditions are
508 extracted from the last iteration of spin up run, and the following model outputs generated after
509 October 1st, 2021.

510 **2.4 Caveats**

511 In the entire study domain, rain gauges are only installed in the Southern Appalachians,
512 specifically in the vicinity of the Cataloochee Creek Basin (Basin 05). However, the rest of the
513 regions are not equipped by raingauge networks, and therefore, no rain gauge bias correction is
514 done for those basins, and the downscaled original dataset StageIV (i.e., STIV_D) is used as input
515 for the IRC method and hydrological simulations in this study.

516 As an important component of the IRC framework, the Lagrangian tracking algorithm is
517 only implemented when hydrological window changes, rather than following model temporal
518 resolution (i.e., 5 minutes), due to practical computational constraints. Additionally, we do not
519 differentiate peak flow points and recession inflection points between simulations and observations
520 when classifying hydrological flow regimes/windows, and consistently use observations delineate
521 hydrological windows simply because 1) particle locations are inherently much more uncertain
522 when simulation time is getting longer partially due to numerical truncation errors and grid-based

523 abruptly-changing velocity fields used in the Lagrangian tracking algorithm, and 2) the
524 computational costs of the tracking algorithm. Very short travel times (i.e., <15 minutes) are
525 ignored because of temporal resolution restrictions from streamflow observations. A systematic
526 use of 24 hours for event total duration is imposed in this work to reduce excessive tracking
527 workload, which might be problematic for events with very long and heavy tails, though not
528 common for flash flood events in headwater basins.

529 The IRC-ICC recursive framework allows us to quantify QPE uncertainties more
530 realistically by improving initial soil moisture estimation, and this framework is numerically
531 efficient in terms of reaching hydrological equilibrium state within 3-5 iterations. In this work, the
532 stable state of IRC-ICC is reached when the KGE changes are bound by 0.05.

533 **3. Results and Discussion**

534 **3.1 Rain gauge Bias Correction**

535 The climatologically corrected $STIV_{DBKC}$ fields have a significantly accurate diurnal cycle
536 compared to only event-scale bias-corrected $STIV_{DBK}$. This process is illustrated in Figure 5 for
537 one rain gauge from each side of the ridges (eastern side: left panel; western side: right panel) in
538 the Southern Appalachians.

539

540 <Figure 5 here please>

541

542 Original $StageIV_D$ show higher biases over the western ridges (e.g., right panel) for all hours of
543 day, illustrating the difficulties of capturing seeder-feeder enhancement of low-level precipitation

544 systems (Duan and Barros, 2017). Also, the mid-day dry bias has been a problem for radar
545 measurements in this region. (e.g., Barros and Arulraj, 2019). Results show that StageIV_{DBKC}
546 datasets capture precipitation climatology better with smaller missing detection errors compared
547 to original StageIV. Figure 6 shows the diurnal characteristics of the missing percipitaaion for
548 two raingauge locations for winter season (January-February and March – JFM) using StageIV,
549 and this phenonemon is observed for both the StageIV_D (black) and StageIV_{DBK} (cyan). These
550 missing cases correspond to light rainfall that have small rainfall measurements at rain gauge
551 locations (< 1.5 mm/hr, bottom row). After applying precipitation climatology corrections, the
552 missing issue in StageIV_{DBK} is significantly alleviated and much better results are shown in
553 StageIV_{DBKC} fields (green).

554

555 <Figure 6 here please>

556

557 The seasonal HSS, TS, and RMSE of STIV_{DBKC} are significantly better than those of
558 STIV_D throughout the day using 10-year averages (Figure 7a). It is worth noting with increasing
559 precipitation rate threshold (Figure 7b), threat score does not show decreasing trend, meaning
560 raingauge bias correction for heavy rainfall events works well. Figure 7c shows RMSE
561 performance conditional on rain rate at diurnal and seasonal scales. Overall, the RMSE is generally
562 less than 0.1 mm/hr except in the cold-season morning and late afternoon, which can be partially
563 attributed to snow events because these raingauges are not heated.

564

565 <Figure 7 here please>

566

567 **3.2 Hydrologic Correction**

568 The coupled IRC-ICC was originally developed and applied in Basin 05, the Cataloochee Creek
569 Basin, and an example showing the results from iterations is demonstrated in Figure 8. The
570 notation follows the definition in Figure 3. Note that the $STIV_{DBKC}$ data derived in Section 3.1 are
571 further downscaled to 250m and used for hydrological simulations in this section. For all other
572 basins (except Basin05), rain gauges are not available, and $STIV_D$ data are used instead.

573

574 <Figure 8 here please>

575

576 It is demonstrated that IRC-ICC produces stable results after about 3 to 4 iterations without
577 significant oscillations for this specific extreme flood event. In general, for less significant events,
578 IRC-ICC reaches equilibrium faster (merely three iterations), providing fast and convergent
579 corrections. As explained earlier, the equilibrium state is reached and thus IRC-ICC is stopped
580 when oscillations in simulated KGE are within 0.05, and then IRC-ICC is stopped immediately.
581 This study suggests that for most events, three iterations is a good rule of thumb. The difference
582 between the initial 4D (x, y, z, t) rainfall forcing and the final result of the IRC-ICC is the general
583 IRC correction.

584

585 **3.2.1 Systematic Application of IRC-ICC**

586 The IRC-ICC is systematically executed in the 28 basins located in the Appalachians for
587 225 events, and examples are displayed in Figure 9.

588

589 <Figure 9 here please>

590

591 Simulated streamflows generally have better performances in the Northern and Southern
592 Appalachian Mountains (NAM, SAM) compared to the Central (CAM). Specifically, in the Karst
593 region along the interstate border of Virginia and West Virginia in the CAM, for Basins 13 and
594 14, where there are numerous caverns and natural tunnels facilitating fast subsurface flow response,
595 that is, sinking and subterranean streams ([https://www.dcr.virginia.gov/natural-](https://www.dcr.virginia.gov/natural-heritage/vacavetrail)
596 [heritage/vacavetrail](https://docslib.org/doc/2284608/west-virginia-tax-districts-containing-karst-terrain) and [https://docslib.org/doc/2284608/west-virginia-tax-districts-containing-](https://docslib.org/doc/2284608/west-virginia-tax-districts-containing-karst-terrain)
597 [karst-terrain](https://docslib.org/doc/2284608/west-virginia-tax-districts-containing-karst-terrain)). The current version of the DCHM does not have a specific module designed for
598 karst geology and karst hydrological processes. Thus, the IRC-ICC results in these locations are
599 impacted by model structural uncertainty. Here, the advantage of not calibrating model parameters
600 becomes apparent. It would be possible to calibrate model parameters to improve model
601 simulations; however, the physical basis and transferability of the IRC-ICC results would be
602 compromised. The 10 events in Basins 13 and 14 are therefore discarded (example: Figure A43).
603 This point of discussion is highlighted here to reinforce the value of the data set presented in this
604 manuscript for applications with other hydrologic models, including model calibration, where
605 model structural uncertainty is not a primary concern at resolved scales.

606 Event 2021-06-10 in Basin 19 (see Figure A43) is an example of an event with a complex
607 hydrograph (e.g., multiple minor flood peaks around one major flood peak) that requires more
608 hydrological windows (see Figure 3). Subtle changes in the hydrograph shape could be indicative
609 of spatial shifts in runoff production from one tributary to another following the track of storm
610 cells over the basin. Indeed, depending on the weather system and regional topography, the travel

611 velocity of such cells and their life-cycle may require finer spatial and temporal resolution both
612 for the hydrological model and for the tracking algorithm to capture changes in the spatial structure
613 of precipitation, especially in the case of summer thunderstorms. For the systematic production of
614 this data set, a 5-window IRC-ICC framework was applied, including a pre-rising-point segment,
615 rising limb, early recession, and late recession (separated by the recession inflection point).

616

617 **3.2.2 IRC and IRC-ICC Precipitation Corrections**

618 Accumulated rainfall totals per rainfall event are calculated for both the IRC-only product
619 and post IRC-ICC products. Subsequently, these rainfall totals are directly compared against
620 original product $STIV_{DBKC}$. Examples are shown in Figure 10, categorized by seasons in the
621 Cataloochee Creek Basin (Basin05). Again, the warm season is defined as April 1st to September
622 30th, and the remaining events are defined as the cold season, with only liquid precipitation events
623 studied in this work.

624

625 <Figure 10 here please>

626

627 The original QPE (**a1** and **b1**) shows abrupt changes in rainfall intensity, which is a
628 common issue of radar observations at high spatial resolution. On the contrary, the IRC-corrected
629 precipitation maps demonstrate precipitation features aligning with landform, showing strong
630 spatial precipitation gradients along ridges and adjacent valleys (examples are listed in Figure A32).

631 The spatial correlation between orographic precipitation and topography is observed across all
632 mountain ranges, including the Appalachians (e.g., Konrad II, 1994; Smith et al., 2011; Wolvin et

633 al., 2024). Note the dark blue colors in Figure 10 corresponding to very low precipitation near the
634 basin outlet are an artifact of the IRC tied to very short travel times that cannot be fully resolved
635 even at fine scales of 250m and 5minutes. However, these artifacts are much reduced for the IRC-
636 ICC due to the reduction of uncertainty in initial conditions, as shown for the 2009-10-14, 2009-
637 04-20, and 2013-04-12 events because of overall basin-wide travel time improvement. It is worth
638 noting that these three events are relatively mild events, indicating a larger impact of IC on
639 relatively less extreme events because of the critical role of IC in runoff generation mechanisms
640 and travel times distributions. Thus, the extreme event precipitation product obtained from IRC-
641 ICC is the data set recommended for applications with other hydrologic models.

642

643 3.2.3 Precipitation and Hydrologic [Skill Metrics Statistics](#)

644 Event-total precipitation maps are calculated for each basin and event, and basin-scale
645 precipitation statistics (e.g., mean and standard deviation) are derived for each event-total
646 precipitation map. These statistics are plotted in Figure 11, and subregions are separated by vertical
647 black lines. Basins 01 to 11 are located in the SAM, Basins 12 to 20 are located in the CAM, and
648 Basins 21 to 30 are located in the NAM. Basins 13 and 14 are not included in the statistics.

649 <Figure 11 here please>

650 It is clearly demonstrated that the change in the mean (i.e., basin-averaged event total QPE)
651 is relatively small (from 36.10mm to 38.07mm) compared to the change in the standard deviation
652 (from 6.63mm to 14.08mm) after the application of IRC-ICC. The small standard deviation of the
653 original QPE suggests that the original QPE data are spatially tightly clustered with low variability
654 (see Figure 10a for boxy rainfall features), while the larger standard deviation post-IRC-ICC

655 indicates spatial variability is enhanced, which is highlighted by the terrain-aligned precipitation
656 features in Figure 10c. The relatively small change in the mean indicates that the original input
657 precipitation (i.e., StageIV_{DBKC} for Basin 05, and StageIV_D for the remaining basins) does not
658 contain significant unconditional systematic biases across basins and events, which would lead to
659 consistent positive or negative flood volume errors. As an exception, it is worth noting that the
660 standard deviation of Basin 05 events does not change significantly after the IRC-ICC compared
661 to other basins and events because rain gauge corrections from the IPHEX network are employed
662 in Basin 05 but not anywhere else. It can never be overly emphasized that even after rain gauge
663 bias correction, essentially a point-scale correction method, the resulting flood hydrograph exhibits
664 significant water budget closure errors (see Figure 12 for more discussion) on account of the high
665 heterogeneous nature of QPE in complex terrain.

666 The hydrologic statistics described in Table 1 using all studied events are plotted in Figure
667 12.

668 <Figure 12 here please>

669 Figure 12 shows that the median KGE across events is improved from 0.36, 0.39, 0.27 to
670 0.89, 0.74, 0.84 for SAM, CAM, and NAM, respectively. It should be pointed out that QPE
671 changes for Basin 05 events (event numbers 55 to 108) are important for improving water budget
672 closure, albeit small in magnitude compared to other events in other basins, as shown in Figure 11
673 and 12, and yet critical to capture the complex precipitation heterogeneity in complex terrain to
674 close the water budget. The results for Basin 05 illustrate the limitations of rain gauge-based bias
675 corrections in complex terrain in general. The relatively small improvement shown in the CAM is
676 partially attributed to the fact that DCHM does not have a proper representation of subterranean
677 rivers in karst terrain, causing large baseflow errors during hydrograph recession and thus low

678 KGE values. Nevertheless, for flash flood applications, peak flow magnitude, flood flow timing,
679 and event flow volume are the most important forecast objectives, corresponding to the 2nd, 3rd,
680 and 4th horizontal panels in Figure 12. Overall, flood volume error (EV) is controlled within $\pm 10\%$
681 for over 90% of the studied events (the 2nd panel), with the median EV error being less than 5% in
682 the SAM and NAM after IRC-ICC corrections. Flood peak volume (the 3rd panel) is generally
683 controlled within 20%, which is very good for extreme events in regions without ground-based
684 observations except for radars placed far away. This is demonstrated by Tropical Storm Fred on
685 2021-08-17: an event that caused floods in multiple SAM basins, caused five deaths, and resulted
686 in an economic loss of more than 1 billion dollars. Note the KGE for this event is improved to 0.9,
687 and peak timing errors are <30 minutes using IRC-ICC. Timing errors (shown in the 4th subplot)
688 are bounded by ± 60 minutes for the major of the events for post IRC-ICC datasets, though some
689 outliers exist potentially due to complex antecedent land surface physics (e.g., rain on snow) for
690 April events, particularly in the CAM and NAM.

691 Events associated with significant timing errors (more than ± 90 minutes) are investigated
692 in detail. These include the 2023-07-08 event (event number 185) for Basin 27, which is located
693 in New Hampshire (the estimated flood front occurs too early by 2.5 hours). This was a localized
694 summer thunderstorm event, only taking 30 minutes to reach its peak flow. The fast changes in the
695 hydrological regime require much more windows than the current classic 5-window settings used
696 in the IRC-ICC framework. The event on 2022-05-27 (event number 118) in Basin16 located in
697 West Virginia is characterized by a slow rising limb. Note Basin16 is partially located in a complex
698 region with karst features (e.g., sink holes) in the Greenbrier-river valley. Finally, the event 2021-
699 09-22, a complex rainfall system characterized by multiple rain cells passing through the Basin 19
700 quickly (event number 133), requiring smaller hydrological windows to capture highly variable

701 rainfall-runoff responses than the 5-window default IRC-ICC architecture: baseflow segment, pre-
702 rising segment, flood rising limb, early and late recession.

703 Overall, large improvements in QPE are achieved, resulting in hydrological improvements
704 in aspects of peak magnitude, flood total volume and flood front timing. Due to the dependence of
705 IRC-ICC on travel time distributions, it cannot be used when precipitation is missing or there are
706 severe timing errors because of the lack of water travel time trajectories to distribute corrections.
707 From a practical point of view, the QPE IRC-ICC correction is in nature a type of space-time bias
708 correction. The improved QPE data facilitates the development of QPE error models, which is
709 demonstrated by the same authors (e.g., Liao and Barros, 2023), providing a path towards
710 correcting remote-sensing products to support hydrometeorological studies and advancing the
711 calibration of hydrological models with significantly less forcing uncertainty.

712

713 **3.2.4 Independent Verification**

714 As mentioned in the introduction, precipitation measurements are limited in the Appalachians
715 except for the IPHEX rain gauge network (Figure 1). Currently, the NEXRAD radar network
716 remains the widely used precipitation monitoring system in this region in spite of well-documented
717 low radar quality coverage over radar gaps in the mountains. The Multi-Radar/Multi-Sensor
718 (MRMS) product (Zhang et al., 2016), which is developed using NEXRAD radar measurements
719 similar to StageIV, is created at 1km resolution and is used here for independent verification.

720 First, original MRMS data are downscaled to the same resolution as StageIV_D datasets (250m,
721 5min) and used as inputs for DCHM. Hydrological simulations in this section are using the same
722 model configuration and initial model states for the purpose of a meaningful comparison, including

723 the following datasets: MRMS_D, StageIV_D, and IRC-ICC StageIV_D as shown in Figure 13. Figure
724 13a shows that MRMS and StageIV QPE have similar results. Second, the IRC-ICC StageIV_D
725 have generally a good agreement with MRMS_D similar to StageIV_D. However, for some cases,
726 where rainfall is dramatically underestimated by the radar system and KGE values are low, IRC-
727 ICC is shown to provide effective corrections. Otherwise, the IRC-ICC generates physically
728 constrained corrections spatially (see Figure 10), achieving high KGE values for flood simulations.
729 Figure 13b shows the histogram of the KGE values across different rainfall products for all events.
730 Overall, simulated streamflows using MRMS_D and StageIV_D exhibit similar hydrologic
731 performance (the median KGE across events is close to 0.20), on the contrary, post-IRC-ICC
732 StageIV_D produce flood simulations with a median KGE above 0.80.

733

734 **4. Discussion and Future Work**

735 Limitations in this study stem mainly from computational constraints rather than
736 methodology. A default 24-hour flood duration window is imposed, implying that for long-lasting
737 floods, due to significant slow interflow and baseflow contributions, are not considered. The
738 current version of the IRC-ICC framework was built to support flash flood studies and only targets
739 shallow subsurface moisture transport, given the critical importance of shallow soil moisture on
740 the regulation of flood generation and propagation in steep terrain. It is worth noting that for long-
741 lasting rainfall events or regions with relatively flat terrain, slow interflows would become more
742 important in terms of regulating flood timing, flood volume, and post IRC-ICC QPE.

743 While the IRC results could be further optimized if carried out at the same frequency as
744 the model resolution, therefore eliminating any artifacts due to inadequate sampling and updating

745 of travel time distributions, and while there is room to improve the IRC-ICC framework through
746 improved model physics and resolution, utilizing 3D velocity fields to capture the full travel time
747 distributions, and using different models to generate IRC ensembles. to test and calibrate
748 hydrologic models for an intercomparison study, advancing flood forecasting skill, and to support
749 emergency management response.

750

751 **5. Data Availability Statement**

752 The StageIV-IRC dataset at 250 m 5-minute resolution for 26 basins and 215 events is
753 available at: <https://doi.org/10.5281/zenodo.14028866>. (Liao and Barros, 2025c), excluding
754 Basin [07 \(the smallest basin\)](#), [12 \(the largest basin\)](#), [13 \(Karst terrain\)](#), and [14 \(Karst terrain\)](#) based
755 on previous discussion. Associated geographic documentation of the selected basins is also
756 provided via the same link. Initial soil moisture distributions for the studied events are also
757 available in the same Zenodo repository.

758

759 **6. Conclusion**

760 QPE has been an enduring challenge in hydrology, particularly in complex terrain. Ground-
761 based radar QPE is plagued with uncertainties from multiple sources, while rain gauge networks
762 are scarce and suffer from the lack of representativeness in the mountains. To address this grand
763 challenge, we develop a series of corrections from point-scale to watershed-scale encompassing
764 event bias, climatology, and water budget closure: the IRC-ICC framework. To our knowledge,
765 this is the first QPE dataset that meets standard statistical evaluations against point-based

766 measurements where available and meets water budget closure at flood-event scale, consistent
767 with nonlinear rainfall-runoff processes in headwater basins, and achieves superior hydrological
768 performance at sub-hourly.

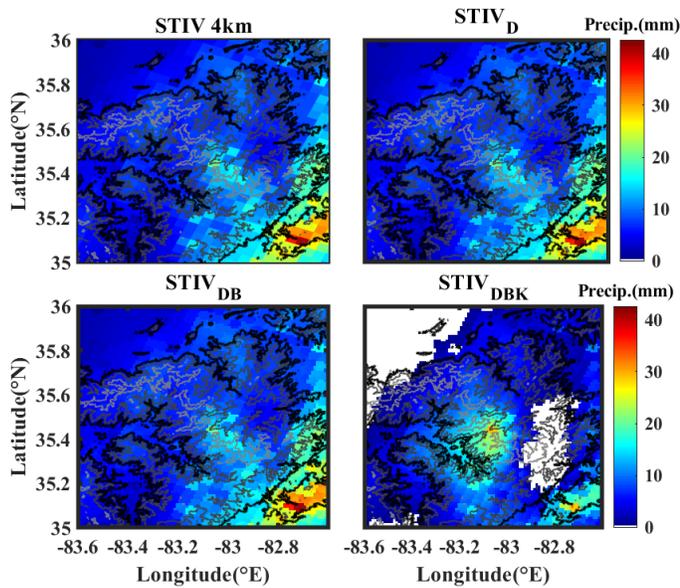
769 The IRC-ICC framework is successfully adopted in 26 mountainous basins (excluding the
770 basins that are heavily overlapped with Karst terrain) in the Appalachians for 215 events with
771 robust success, yielding substantial improvements of streamflow simulation, particularly in terms
772 of flood volume and timing. The tracking algorithm in the IRC-ICC framework is only updated
773 when shifting from one hydrological window to another, but not every time step. With enough
774 computational resources, post-IRC-ICC QPE data should further improve by capturing transient
775 travel time distributions between model time steps.

776 When using the StageIV-IRC product, flood timing errors are controlled with one hour for
777 90% of events, compared to less than 20% when using original StageIV, while the median KGE
778 improved from 0.34 to 0.86 across the events. This change in KGE is achieved by significant
779 changes in the space-time variance of precipitation that in turn impacts the space-time variability
780 of rainfall-runoff processes. Results illustrate the importance of initial conditions for less severe
781 rainfall events, particularly during the beginning of the event, which influences subsequent
782 streamflow simulations. It should be emphasized that physical parameters are not calibrated for
783 any precipitation event in any basin in this work. This physics-based IRC-ICC framework can
784 capture the fundamental physics involved in flash flood events: essentially the fast rainfall-runoff
785 responses in surface and shallow subsurface layers; therefore, skillful hydrologic prediction is
786 achieved without model calibration. Instead, the focus is on getting the forcing right.

787 The IRC-ICC is a general framework that can be incorporated into any distributed
788 hydrological model. Thus, the StageIV-IRC dataset also enables meaningful intercomparison

789 among different radar QPE datasets, providing physics insights into QPE error structure from a
 790 water budget closure perspective, toward improving radar retrievals and to characterize radar-
 791 specific errors related to radar operations at high spatial resolution in the mountains. The
 792 demonstrated success of StageIV-IRC in ungauged basins strongly supports the use of IRC-ICC
 793 in mountainous regions worldwide, where rain gauges are generally not available. Further, this
 794 dataset can be utilized as a reference for building machine learning models (or even deep-learning
 795 models when the number of studied precipitation events is expanded) that can learn the QPE
 796 uncertainties conditional on time of day, weather, climate and geomorphological regimes for both
 797 radar QPE analysis and forecasts, advancing the understanding and quantification of orographic
 798 precipitation uncertainty at high resolution across global mountains.

799 **7. Appendix A**

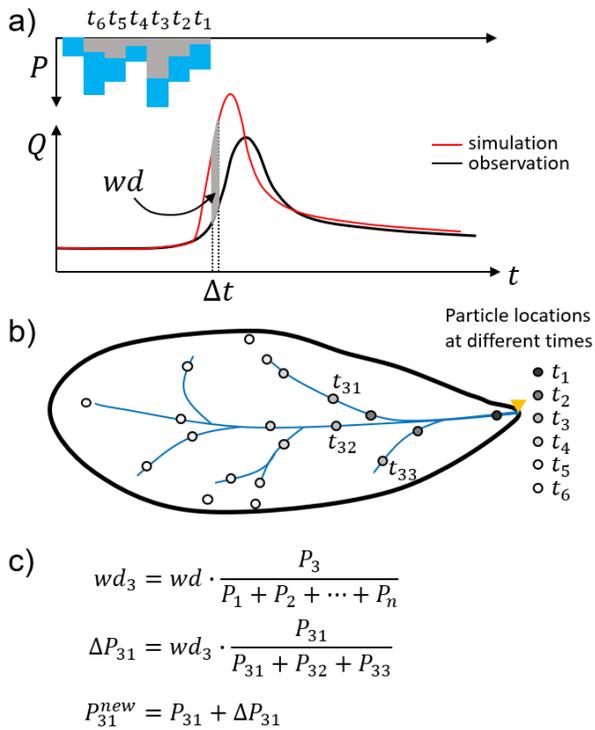


800

801 **Figure A1** - Spatial rainfall fields on 2014-05-15, 06-07 UTC. Rain rates between 0 to 1mm/h are
 802 mapped in white.

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803
 804 The detailed distribution process of water difference (wd) is illustrated in Figure A2+ following
 805 Section 2.3.8.

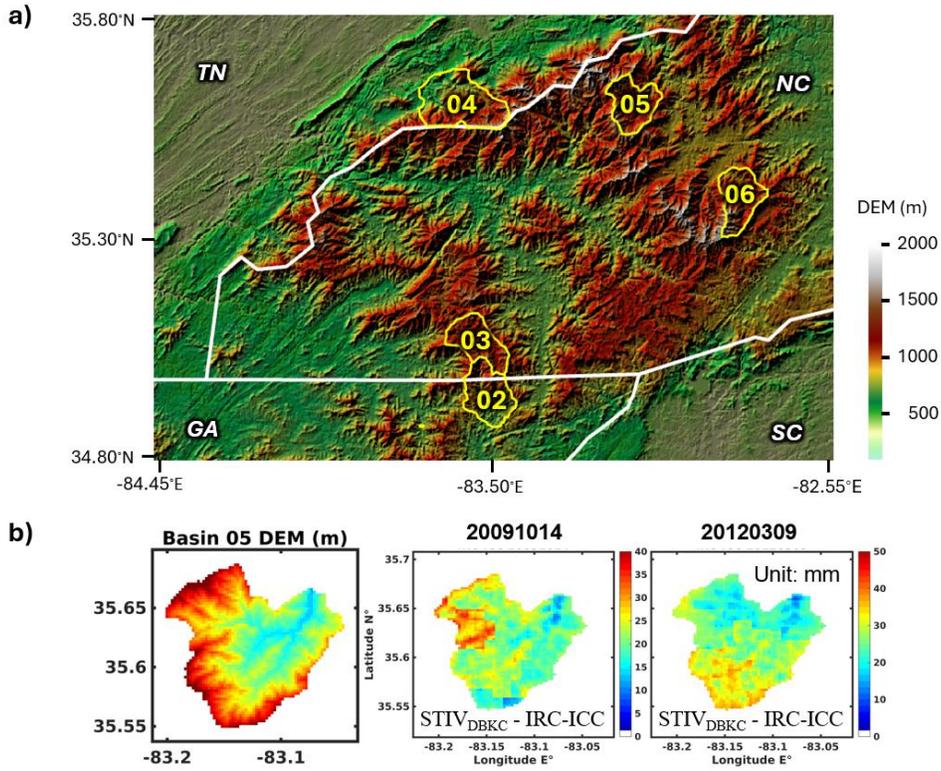


806
 807
 808 **Figure A1-A2** - Schematic depiction of the IRC framework and key mathematical equations. Panel
 809 (a) illustrates the nonlinear relationship between streamflow and precipitation, where *wd*
 810 represents the residual between discharge simulations and observations at the basin outlet. The

811 variation of precipitation in the basin as a function of time is shown by the basin hyetograph in
812 blue. The hyetograph time series (blue) spans the duration of the precipitation event between t_1
813 to t_n . In gray is the hyetograph over the area of interest for panels (b) and (c). To map the
814 streamlines, water particles are launched every time step and their trajectory to the outlet is tracked
815 and saved. Panel (b) shows the source areas of water particles launched at various time steps
816 ($t_1, t_2, \dots, t_6 \dots$) from all locations where runoff is produced, and the particles are tracked until they
817 eventually reach the basin outlet. The streamlines of particles that reach the outlet at the same time
818 are used to distribute the residuals backwards to the runoff source areas where the particles were
819 originally launched (e.g., the three particles t_{31}, t_{32} , and t_{33} that reach the basin outlet at time t_3).
820 Panel (c) shows the algorithm to calculate the rainfall bias correction at location t_{31} due to the
821 residual wd_3 at time t_3 . P_i is basin averaged rainfall at time t_i , and wd_3 is the runoff volume to
822 be corrected at time step t_3 . ΔP_{31} is the precipitation correction for pixel t_{31} , and precipitation
823 amount at pixel t_{31} before and after IRC are denoted by P_{31} and P_{31}^{new} . This figure is adapted from
824 Liao and Barros (2025b).

825
826 A zoom in map of the Southern Appalachians is plotted associated with DEM maps of other basins.
827 A complete set of maps for each individual basin can be requested. Note, the rain gauges used in
828 this study are plotted in Figure 1, and they are primarily near Basin05.

829



830

831 **Figure A32** – A zoom-in map of the Figure 4 for watersheds in the Southern Appalachians (Panel
832 a). The DEM map and examples of rainfall event accumulation of Basin 05 (Panel b) to show
833 rainfall alignment with topography.

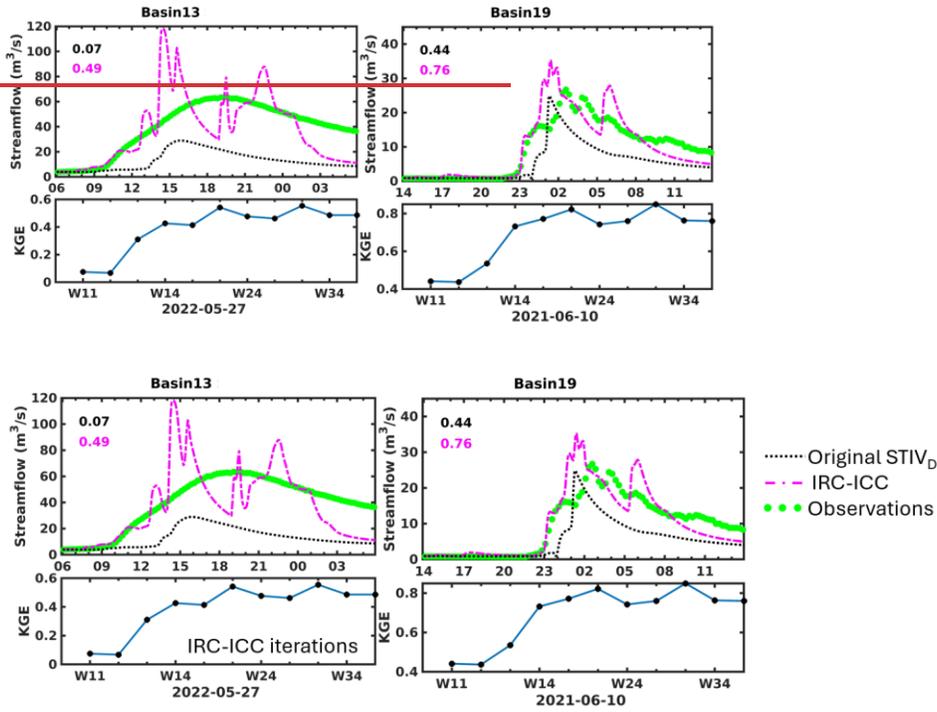
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839

840 **Figure A43** – Examples of the coupled IRC-ICC framework application in Basin 13 and Basin 19
 841 for discussion in the manuscript. KGE values are displayed in the top left corners. Basin 13 is
 842 located in Karst terrain, while the event in Basin 19 is an example with a complex hydrograph.

843

844 **CREDIT AUTHOR STATEMENT**

845 M. Liao: Methodology, Data curation, Writing - original draft, Investigation. A. P. Barros:
 846 Conceptualization, Methodology, Writing - review & editing, Supervision, Project administration,
 847 Funding acquisition.

848 **COMPETING INTERESTS**

849 The authors declare there are no competing interests.

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854

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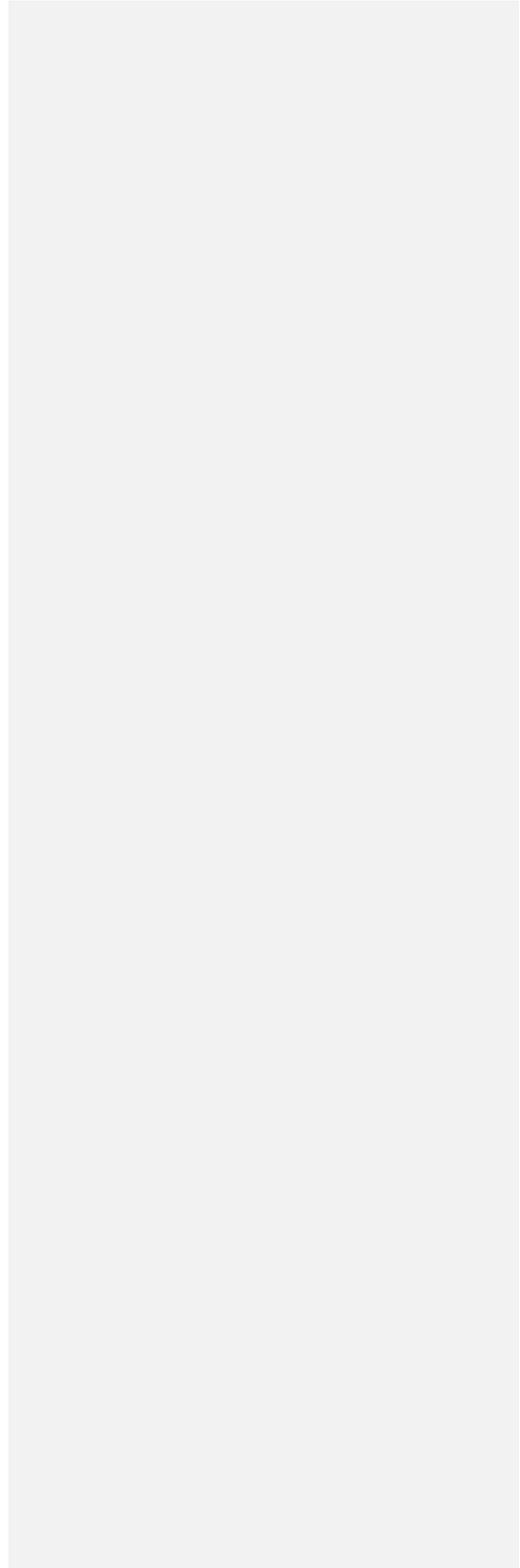
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1191 **LIST OF TABLES**

1192 **Table 1** - Raingauge index and exact locations as illustrated in Figure 1. Two rain gauges
1193 highlighted in bold font are installed at Purchase Knob, a supersite in the inner mountain region.
1194 Locations equipped with more than one raingauge (collocated) are shaded in grey color, and these
1195 collocated raingauges generally differ in tipping sizes. This table is adapted from Liao and Barros
1196 (2019).

1197 **Table 2** - Hydrologic skills used in this work.

1198 **Table 3** - Information table for selected basins and corresponding streamflow gauges used in this
1199 work. This table is adapted from Liao and Barros (2025b). Basin 07 and Basin 12 are the smallest
1200 and largest basins respectively and were removed from further analysis. Red-coded rows represent
1201 basins located in Karst terrain, which are ~~were not included~~ discarded from in the final published
1202 data due to the lack of Karst analysis hydrology processes in the hydrology model and
1203 consequently lesser performance of the IRC in this work.
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1208 **Table 1** – Raingauge index and exact locations as illustrated in Figure 1. Two rain gauges
 1209 highlighted in bold font are installed at Purchase Knob, a supersite in the inner mountain region.
 1210 Locations equipped with more than one raingauge (collocated) are shaded in grey color, and these
 1211 collocated raingauges generally differ in tipping sizes. This table is adapted from Liao and Barros
 1212 (2019).

NO.	Site ID.	Latitude	Longitude	Elevation (m)
01	RG 001	35.398	-82.913	1156
02	RG 002	35.417	-82.971	1731
03	RG 003	35.384	-82.916	1609
04	RG 004	35.368	-82.990	1922
05	RG 005	35.408	-82.964	1520
06	RG 008	35.382	-82.973	1737
07	RG 010	35.456	-82.946	1478
08	RG 100	35.586	-83.072	1495
09	RG 100T	35.587	-83.064	1485
10	RG 101	35.575	-83.088	1520
11	RG 102	35.563	-83.103	1635
12	RG 103	35.553	-83.117	1688
13	RG 104	35.554	-83.088	1584
14	RG 106	35.432	-83.029	1210
15	RG 109	35.495	-83.040	1500
16	RG 110	35.548	-83.148	1563
17	RG 300	35.726	-83.216	1558
18	RG 301	35.705	-83.255	2003
19	RG 302	35.721	-83.246	1860
20	RG 303PK	35.586	-83.072	1495
21	RG 303S	35.762	-83.162	1490
22	RG 304	35.670	-83.182	1820
23	RG 305	35.691	-83.131	1630
24	RG 306	35.745	-83.171	1536
25	RG 307	35.651	-83.199	1624
26	RG 308	35.730	-83.182	1471
27	RG 309	35.682	-83.150	1604
28	RG 310	35.702	-83.122	1756
29	RG 311	35.765	-83.140	1036
30	RG 400	35.702	-83.122	1756
31	RG 401	35.651	-83.199	1624
32	RG 402	35.721	-83.246	1860
33	RG 403	35.517	-83.101	925
34	RG 407	35.517	-83.101	925

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1215 **Table 2:** Hydrologic skills used in this work.

Notation	Information	Reference
KGE	Kling-Gupta efficiency	Eq. (19) /Gupta et al. (2009)
EV	Relative error in flood volume	Eq. (20)
EPT	Error in peak flow timing	Flood front timing differences
EPV	Relative Error in maximum flow rate	Eq. (21)

1216

1217 **Table 3** – Information table for initially selected basins and corresponding streamflow gauges used
 1218 in this work. This table is adapted from Liao and Barros (2025b). Basin 07 and Basin 12 are the
 1219 smallest and largest basins and were removed from further analysis. Red-coded rows represent
 1220 basins located in Karst terrain, which were discarded from the final published data due to the lack
 1221 of Karst analysis in this work.

Basin index	USGS Gauge ID	Drainage area (km ²)	Basin highest elevation (m)	Basin relief (m)	Location
1	3544970	118.7	1442	847	GA
2	2178400	176.1	1629	1051	GA
3	3504000	149.9	1667	1032	NC
4	3497300	317.6	1999	1651	TN
5	3460000	148.1	1879	1174	NC
6	3456500	152.8	1873	1157	NC
7	3450000	15.8	1934	1047	NC
8	344894205	41.3	1995	1221	NC
9	3463300	134.3	1989	1425	NC
10	3400500	234.7	1257	1257	KY
11	3479000	283.3	1772	1216	NC
12	3161000	611.8	1834	1009	NC
13	3182700	447.3	1111	717	WV
14	2011460	194.4	1388	763	VA
15	1620500	54.5	1321	712	VA
16	3180500	426.8	1416	621	WV
17	3068800	437.1	1471	908	WV
18	1595000	234.8	1230	560	MD
19	1595300	130.3	1069	712	WV
20	1544500	445.9	765	457	PA
21	1422747	81.4	766	394	NY
22	1415000	106.8	1019	636	NY
23	1413398	152.8	1094	754	NY
24	13621955	41.7	1074	717	NY
25	1421610	51.3	970	497	NY
26	1074520	389.4	1582	1582	NH
27	10642505	294.9	1895	1693	NH
28	1137500	300.3	1894	1546	NH
29	1133000	183.2	975	719	VT
30	1055000	334.1	1143	975	MAINE

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1224 **LIST OF FIGURES**

1225 **Figure 1** - Map of IPHEX (Barros et al., 2014) ground-based observations in the Southern
1226 Appalachians. Raingauge is denoted as a character string starting with three-digit number
1227 potentially followed by extra letters; locations started with a letter P represent disdrometers. The
1228 basic information regarding these stations is listed in Table 1. This figure is adapted from Liao and
1229 Barros (2019).

1230 **Figure 2** – Workflow to generate the product $STIV_{DBKC}$.

1231 **Figure 3** – An illustration of the structure of IRC, ICC and the coupled IRC-ICC framework
1232 including **a)** the residual hydrograph between the observed and simulated discharge, with the
1233 discharge water difference $wd(t)$ being distributed across the time window T ; **b)** Example of travel
1234 time distribution $TT(t)$ and map (inset) illustrating a hypothetical distribution of runoff source
1235 areas (in red, $ns=3$) with travel time x_2 contributing to streamflow at time t , meaning that at time
1236 $t-x_2$ there are three pixels ($ns=3$) generating runoff that reaches the outlet at time t . T is the time
1237 window over which runoff source areas with $TT < T$ are mapped and the inverse rainfall correction
1238 (IRC) are applied; **c)** Example of IRC windows guided by timescales of dominant hydrological
1239 processes. The first window solely covers the initial streamflow conditions before the target event.
1240 The second window depicts the early rising limb of the hydrograph. The third window captures
1241 the steep rising limb of the hydrograph until it reaches the peak flow. The fourth and fifth windows
1242 correspond to interflow-dominant and baseflow-dominant stages of the recession curve
1243 respectively, separated by the recession inflection point; **d)** A schematic drawing that shows
1244 different characteristic timings in a hydrograph with the implementation of the Initial Condition
1245 Correction (ICC) strategy. Specifically, T_r^* and T_r represent the timing of flood front in simulations
1246 and observations, respectively. T_p is the timing of observed maximum flood. The inflection point
1247 of the recession curve of the observations is denoted as T_i . Flow differences at t_1 and t_2 are denoted
1248 as ΔS_1 and ΔS_2 respectively for the purpose of discussion. P , Q and IC represent precipitation,
1249 flow discharge and initial condition, respectively; **e)** The implemented framework in this work
1250 consisting of ICC and IRC. This figure is adapted from Liao and Barros (2022, 2025b).

1251 **Figure 4** – Map of the Continental United States (CONUS) and headwater basins studied in this
1252 work. Basin information is available in Table 3. Sub-regions are delineated as the following for
1253 discussion purposes only: Northern, Central and Southern Appalachian Mountains (NAM, Basin
1254 21-30; CAM, Basin 13-20; SAM, Basin 01-11). Basins 07 and 12, marked with red symbols, were
1255 discarded due to basin size criteria. Basins 13 and 14 with blue symbols were discarded due to the
1256 importance of karst hydrology processes that are not represented in the hydrology model used to
1257 conduct the IRC. This figure is adapted from Liao and Barros (2025b). ~~Map of the Continental~~
1258 ~~United States (CONUS) and headwater basins studied in this work. Basin information is available~~
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1260 ~~Central and Southern Appalachian Mountains (NAM, Basin 21-30; CAM, Basin 13-20; SAM,~~
1261 ~~Basin 01-11). This figure is adapted from Liao and Barros (2025b).~~

1262 **Figure 5** - Examples of raingauge measurements showing the diurnal cycle of different seasons at
1263 different locations: Left panel – raingauge RG008 located in the eastern ridges for the Summer
1264 (JAS: July-August-September) season. Right panel – raingauge RG302 located in the western

1265 ridges for the Spring (AMJ; April-May-June) season. Rain gauge measurements (blue);
1266 StageIV_{DBK} (black); StageIV_{DBKC} (green). This figure is from Liao and Barros (2019).

1267 **Figure 6** – Top row – The diurnal cycle of missing precipitation at RG003 (Eastern ridges) and
1268 RG103 (Inner regions) for January-February-March (JFM) using various products. Bottom row-
1269 corresponding rain gauge climatology (blue). StageIV_D (black); StageIV_{DBK} (cyan); StageIV_{DBKC}
1270 (green). This figure is from Liao and Barros (2019).

1271 **Figure 7** – Statistical evaluation summary for winter precipitation (JFM, January, February, and
1272 March): a) Diurnal cycle of mean HSS and TS statistics including all rain gauges calculated using
1273 all data from 2008 to 2017: STIV_D (black) and STIV_{DBKC} (green); b) HSS and TS statistics
1274 calculated using different rain rate thresholds over the same 10-year period; c) Diurnal cycle of
1275 rain rate RMSE at seasonal-scale, and its dependence on observed rainfall rate. This figure is from
1276 Liao and Barros (2019).

1277 **Figure 8** – The IRC-ICC performance in Basin05 as an example for the 2017-10-23 event
1278 (Basin05: Cataloochee Creek Basin, NC). This event is part of the 2017 Hurricane Nate. This
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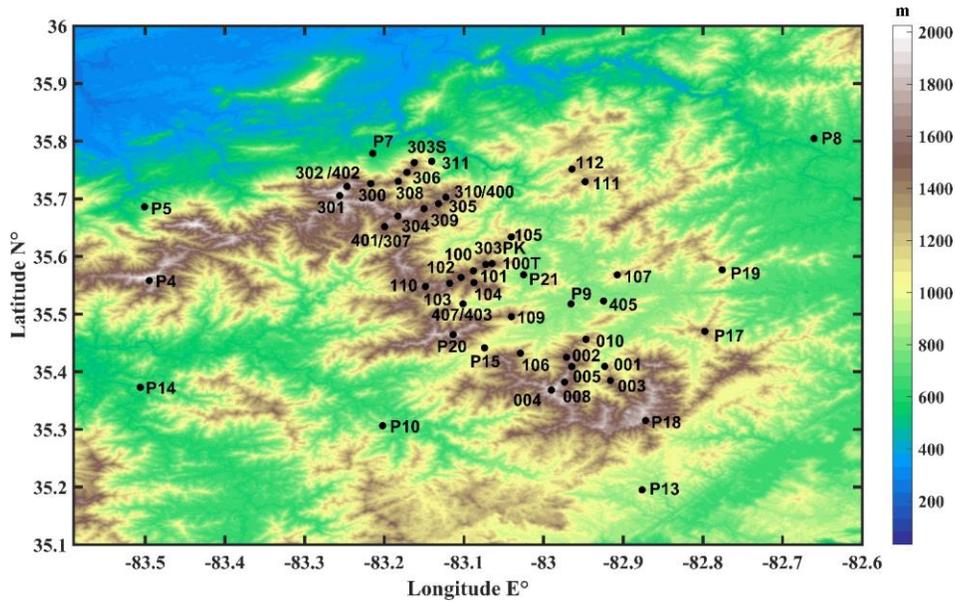
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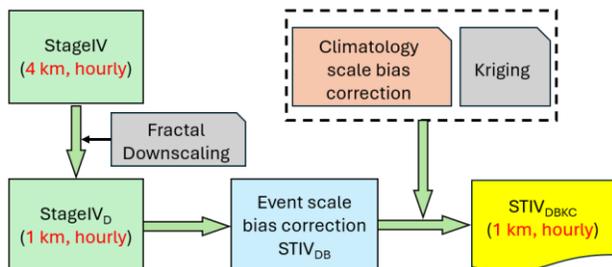
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1314 **Figure 1** - Map of IPHEX (Barros et al., 2014) ground-based observations in the Southern
1315 Appalachians. Rain gauge is denoted as a character string starting with three-digit number
1316 potentially followed by extra letters; locations started with a letter P represent disdrometers. The
1317 basic information regarding these stations is listed in Table 1. This figure is adapted from Liao and
1318 Barros (2019).

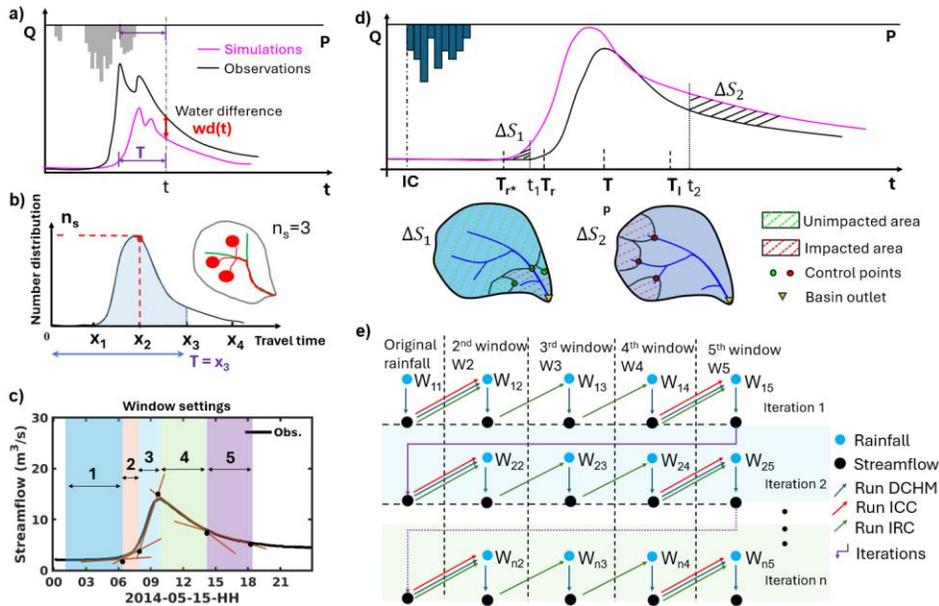
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1321 **Figure 2** – Workflow to generate the product STIV_{DBKC}.

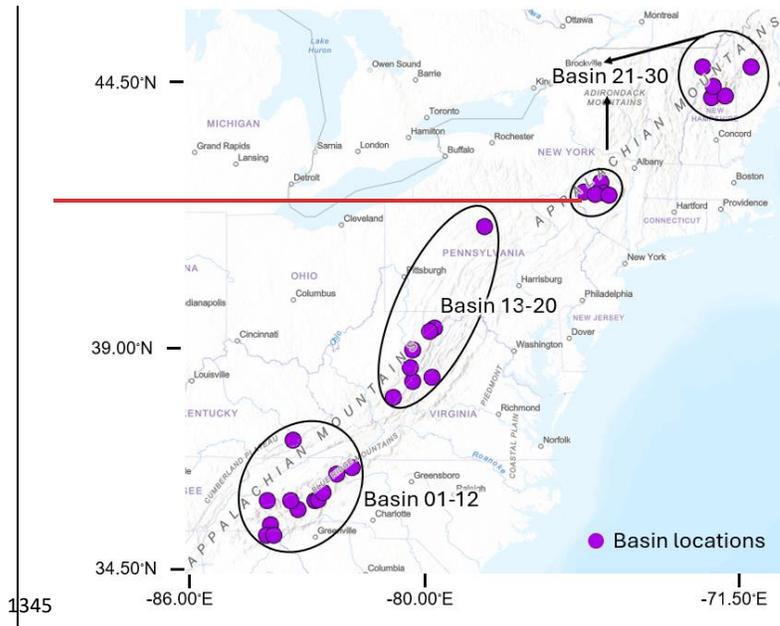
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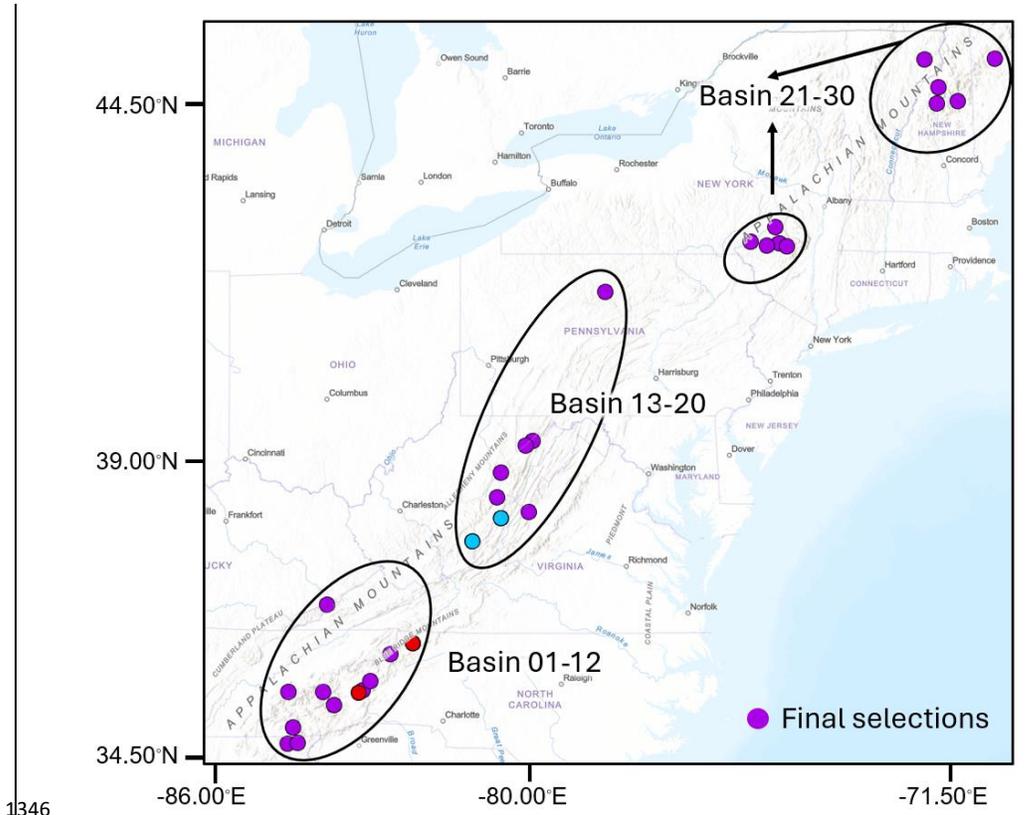
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1324 **Figure 3** – An illustration of the structure of IRC, ICC and the coupled IRC-ICC framework
 1325 including **a)** the residual hydrograph between the observed and simulated discharge, with the
 1326 discharge water difference $wd(t)$ being distributed across the time window T ; **b)** Example of travel
 1327 time distribution $TT(t)$ and map (inset) illustrating a hypothetical distribution of runoff source
 1328 areas (in red, $n_s=3$) with travel time x_2 contributing to streamflow at time t , meaning that at time
 1329 $t-x_2$ there are three pixels ($n_s=3$) generating runoff that reaches the outlet at time t . T is the time
 1330 window over which runoff source areas with $TT < T$ are mapped and the inverse rainfall correction
 1331 (IRC) are applied; **c)** Example of IRC windows guided by timescales of dominant hydrological
 1332 processes. The first window solely covers the initial streamflow conditions before the target event.
 1333 The second window depicts the early rising limb of the hydrograph. The third window captures
 1334 the steep rising limb of the hydrograph until it reaches the peak flow. The fourth and fifth windows
 1335 correspond to interflow-dominant and baseflow-dominant stages of the recession curve
 1336 respectively, separated by the recession inflection point; **d)** A schematic drawing that shows
 1337 different characteristic timings in a hydrograph with the implementation of the Initial Condition
 1338 Correction (ICC) strategy. Specifically, T_{r^*} and T_r represent the timing of flood front in simulations
 1339 and observations, respectively. T_p is the timing of observed maximum flood. The inflection point
 1340 of the recession curve of the observations is denoted as T_1 . Flow differences at t_1 and t_2 are
 1341 denoted as ΔS_1 and ΔS_2 respectively for the purpose of discussion. P , Q and IC represent precipitation,
 1342 flow discharge and initial condition, respectively; **e)** The implemented framework in this work
 1343 consisting of ICC and IRC. This figure is adapted from Liao and Barros (2022, 2025b).

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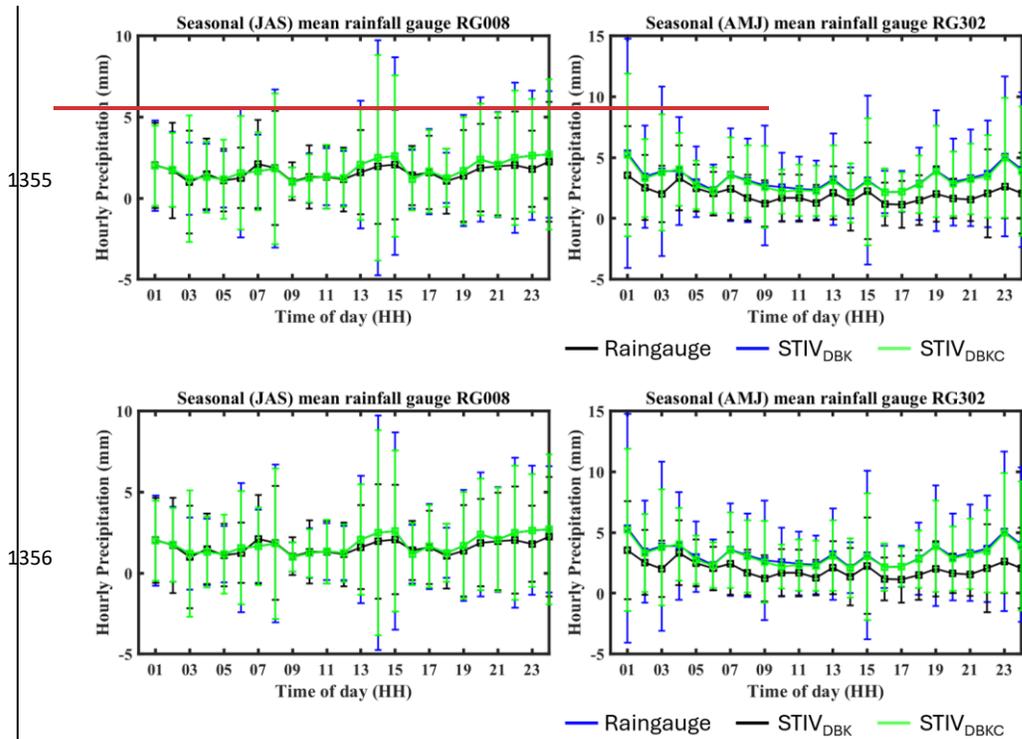


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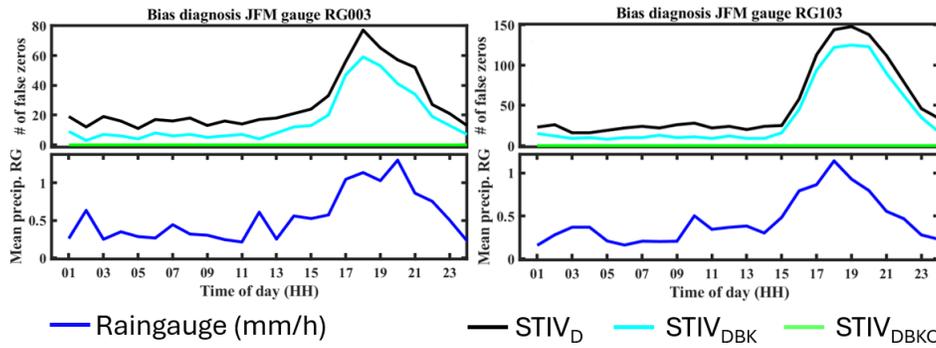
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 1347 **Figure 4** – Map of the Continental United States (CONUS) and headwater basins studied in this
 1348 work. Basin information is available in Table 3. Sub-regions are delineated as the following for
 1349 discussion purposes only: Northern, Central and Southern Appalachian Mountains (NAM, Basin
 1350 21-30; CAM, Basin 13-20; SAM, Basin 01-11). Basins 07 and 12, marked with red symbols, were
 1351 discarded due to basin size criteria. Basins 13 and 14 with blue symbols were discarded due to the
 1352 importance of karst hydrology processes that are not represented in the hydrology model used to
 1353 conduct the IRC. This figure is adapted from Liao and Barros (2025b).

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1357 **Figure 5** – Examples of raingauge measurements showing the diurnal cycle of different seasons
 1358 at different locations: Left panel – raingauge RG008 located in the eastern ridges for the Summer
 1359 (JAS: July-August-September) season. Right panel – raingauge RG302 located in the western
 1360 ridges for the Spring (AMJ; April-May-June) season. Rain gauge measurements (blue);
 1361 StageIV_{DBK} (black); StageIV_{DBKc} (green). This figure is from Liao and Barros (2019).

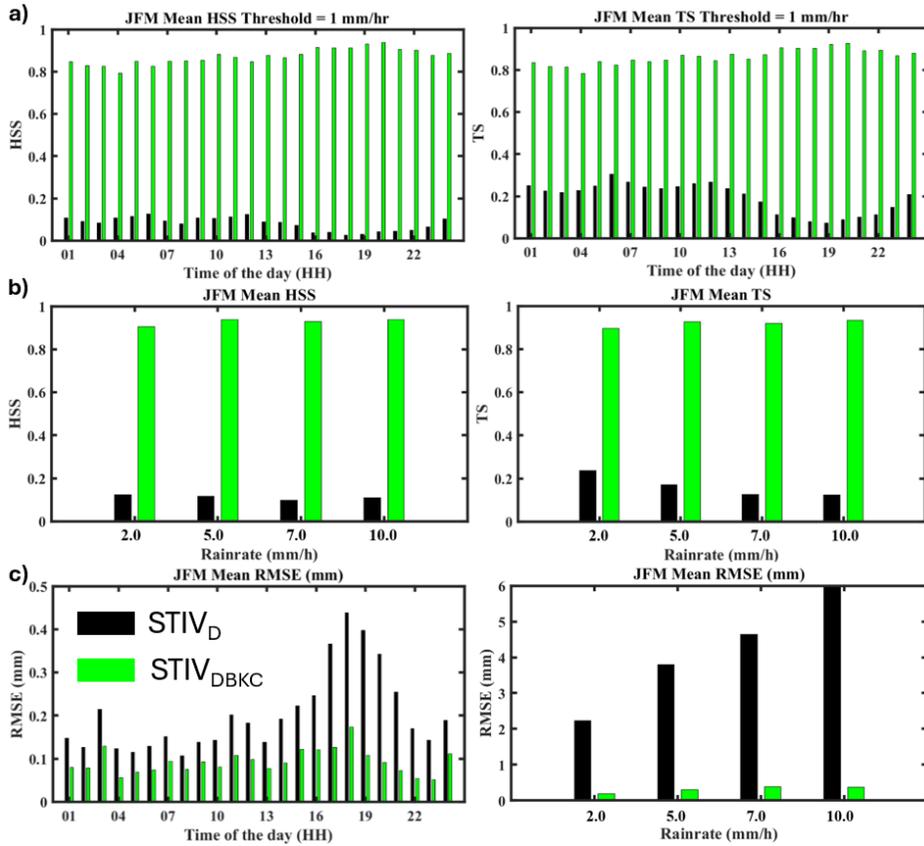
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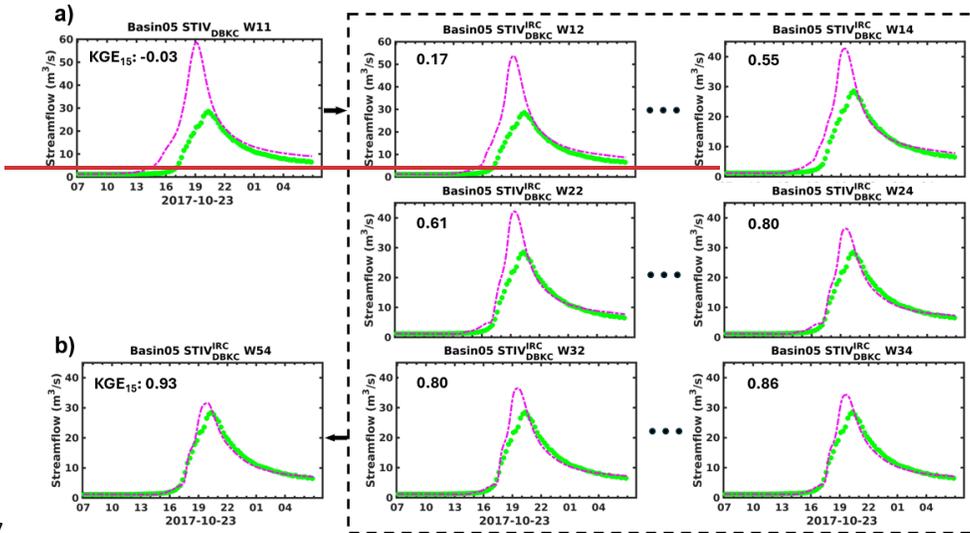
1364 **Figure 6** –Top row – The diurnal cycle of missing precipitation at RG003 (Eastern ridges) and
 1365 RG103 (Inner regions) for January-February-March (JFM) using various products. Bottom row-
 1366 corresponding rain gauge climatology (blue). StageIV_D (black); StageIV_{DBK} (cyan); StageIV_{DBKc}
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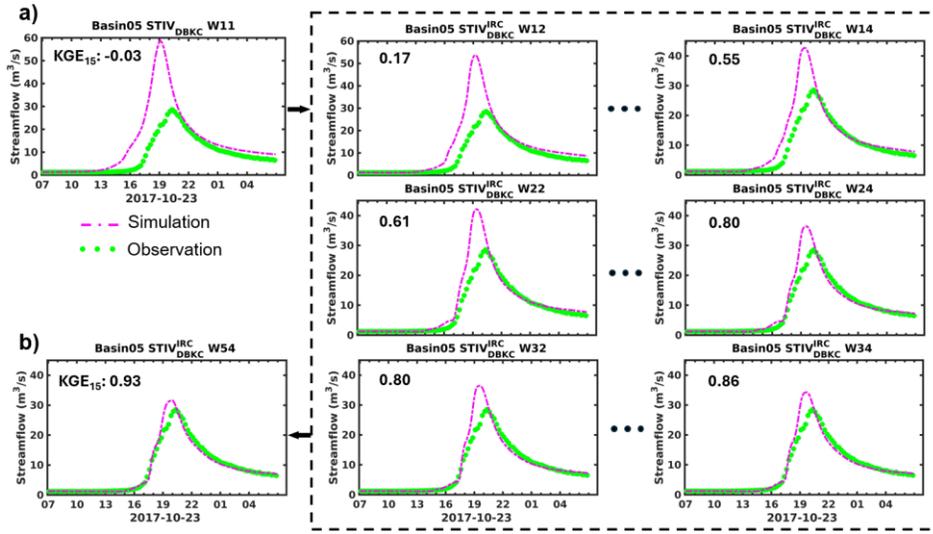


1371 **Figure 7** – Statistical evaluation summary for winter precipitation (JFM, January, February, and
 1372 March): a) Diurnal cycle of mean HSS and TS statistics including all rain gauges calculated using
 1373 all data from 2008 to 2017: STIV_D (black) and STIV_{DBKC} (green); b) HSS and TS statistics
 1374 calculated using different rain rate thresholds over the same 10-year period; c) Diurnal cycle of
 1375 rain rate RMSE at seasonal-scale, and its dependence on observed rainfall rate. This figure is from
 1376 Liao and Barros (2019).

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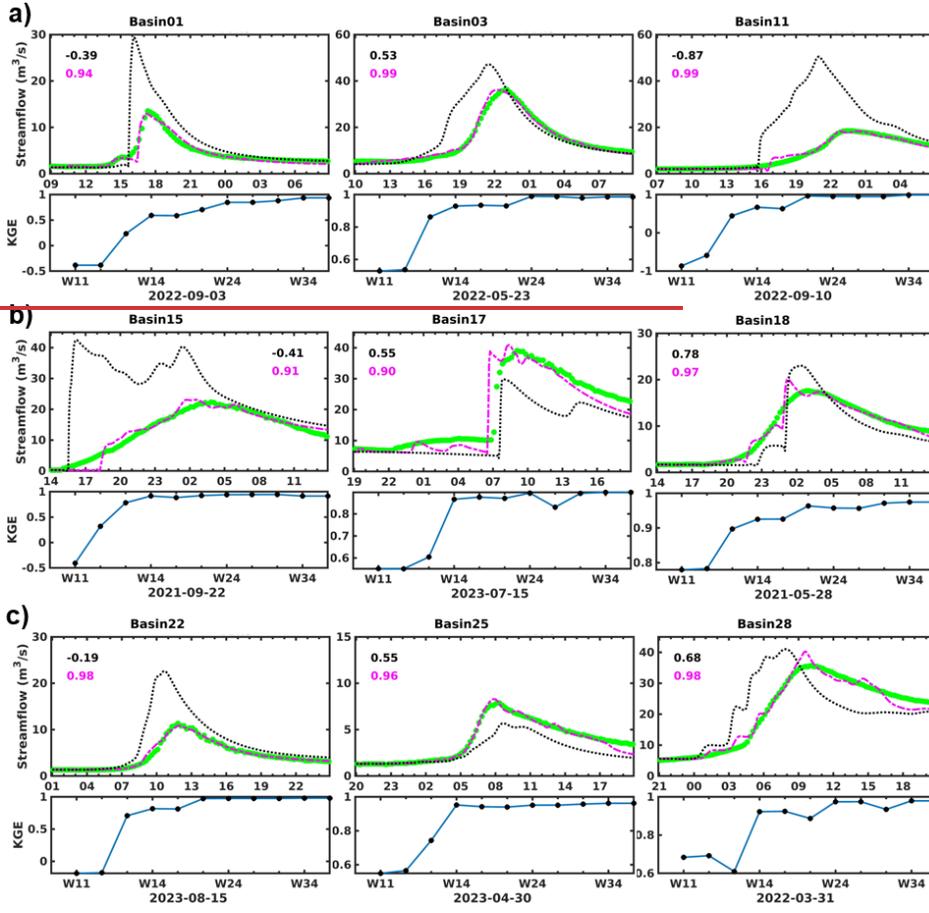
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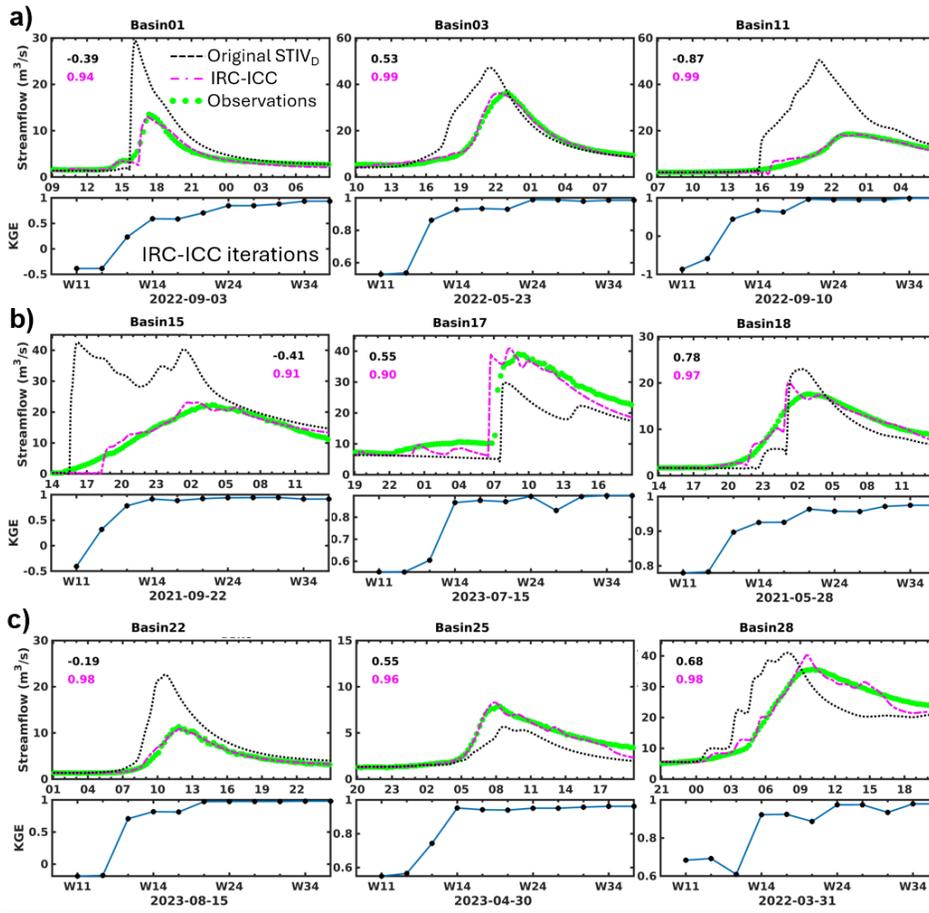


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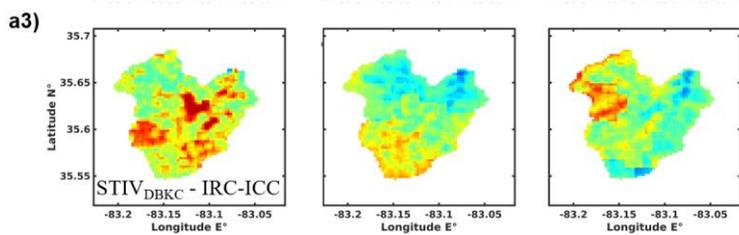
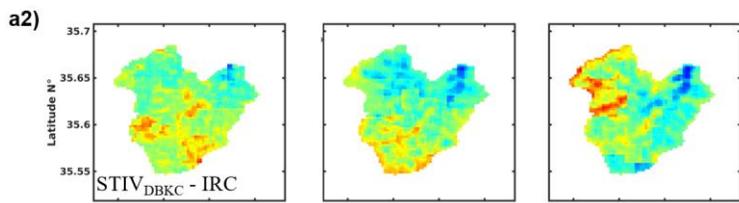
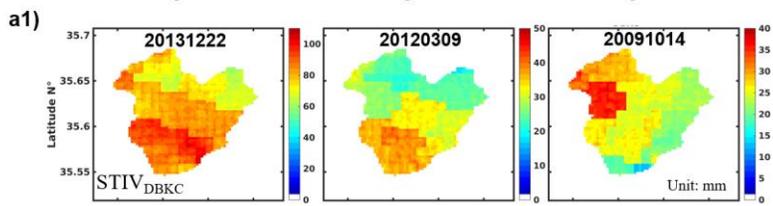
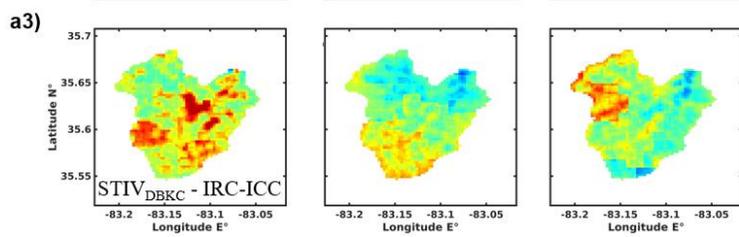
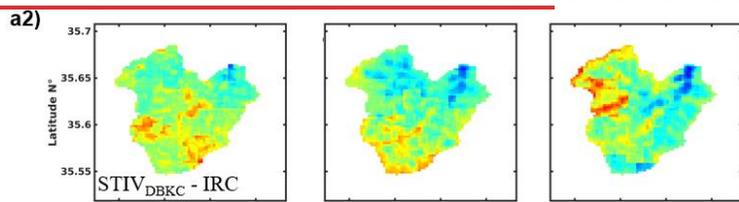
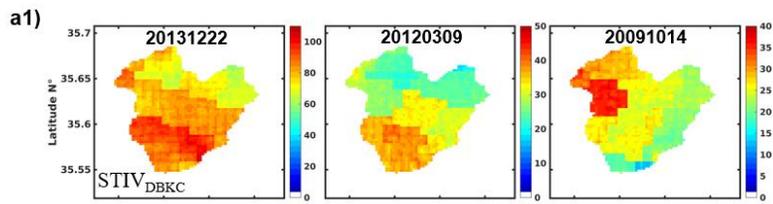
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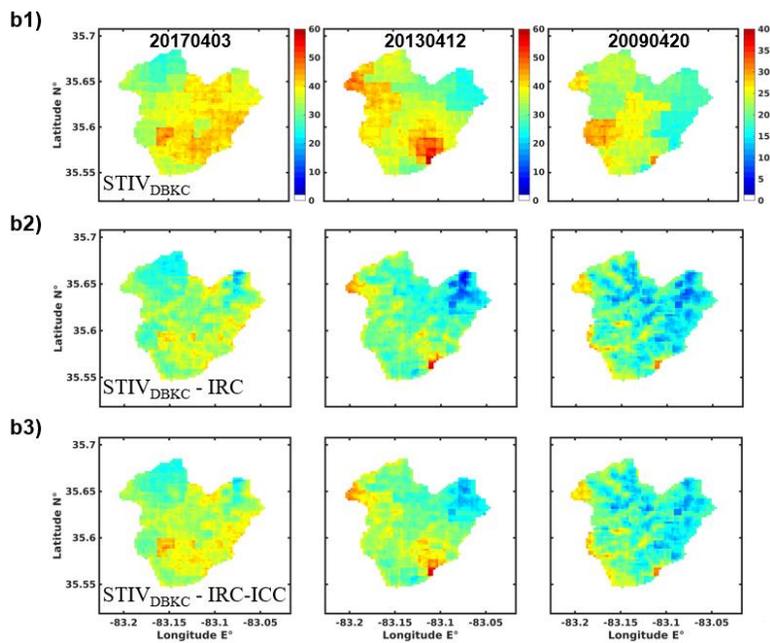




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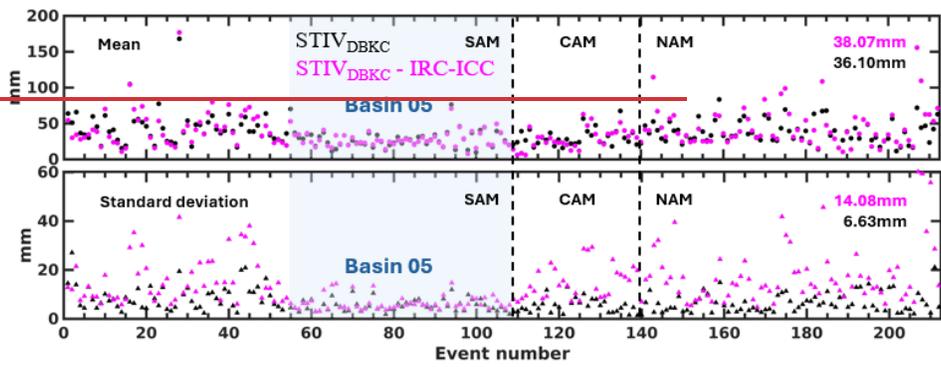




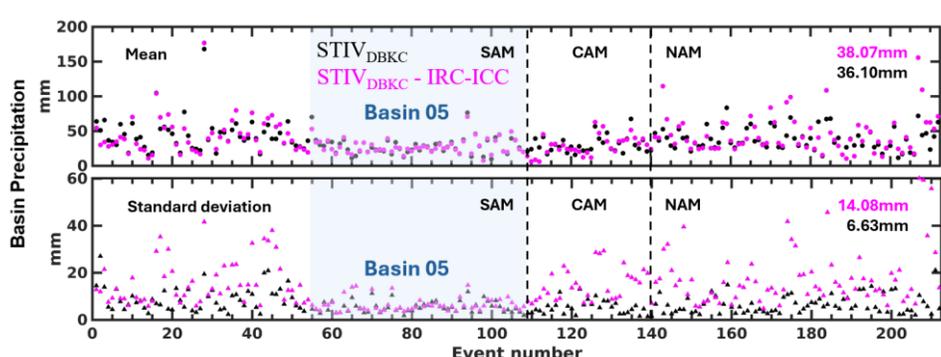
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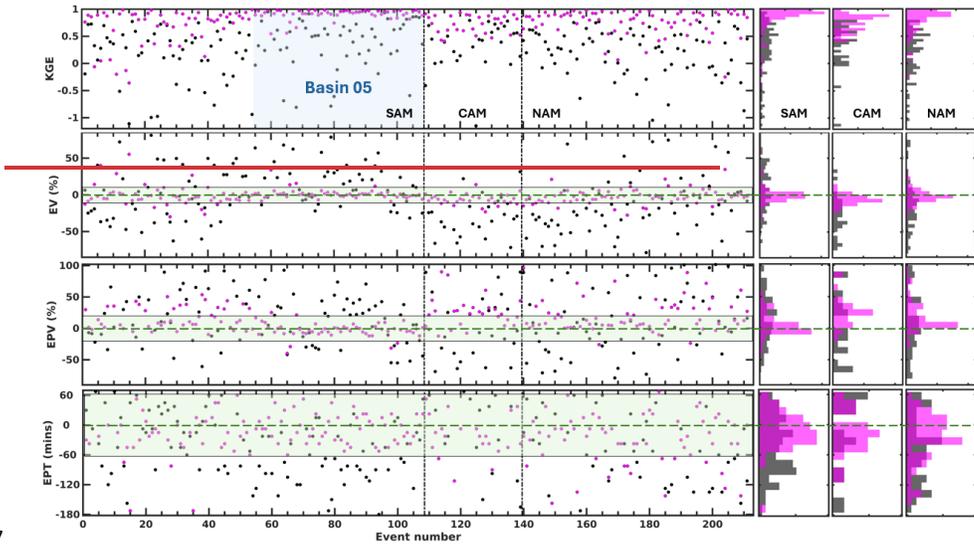


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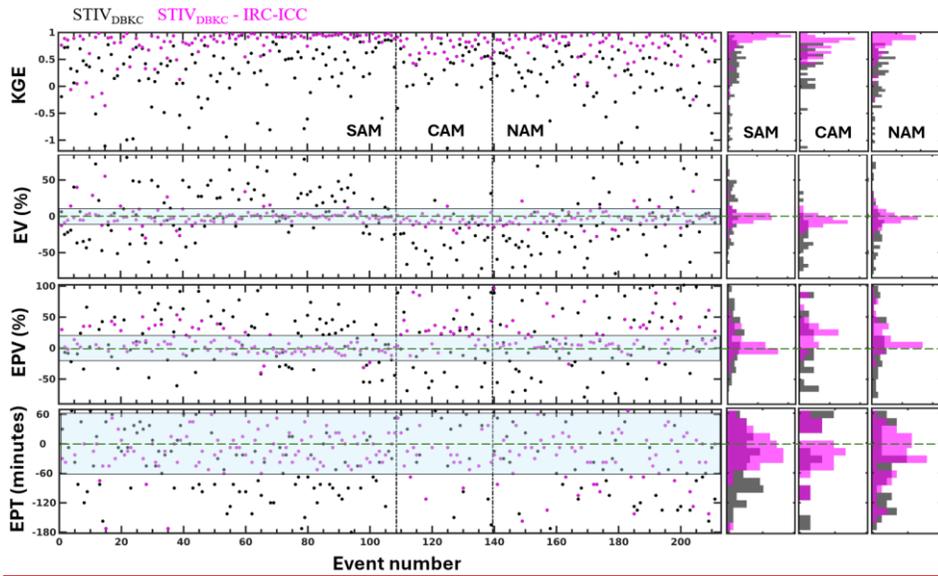
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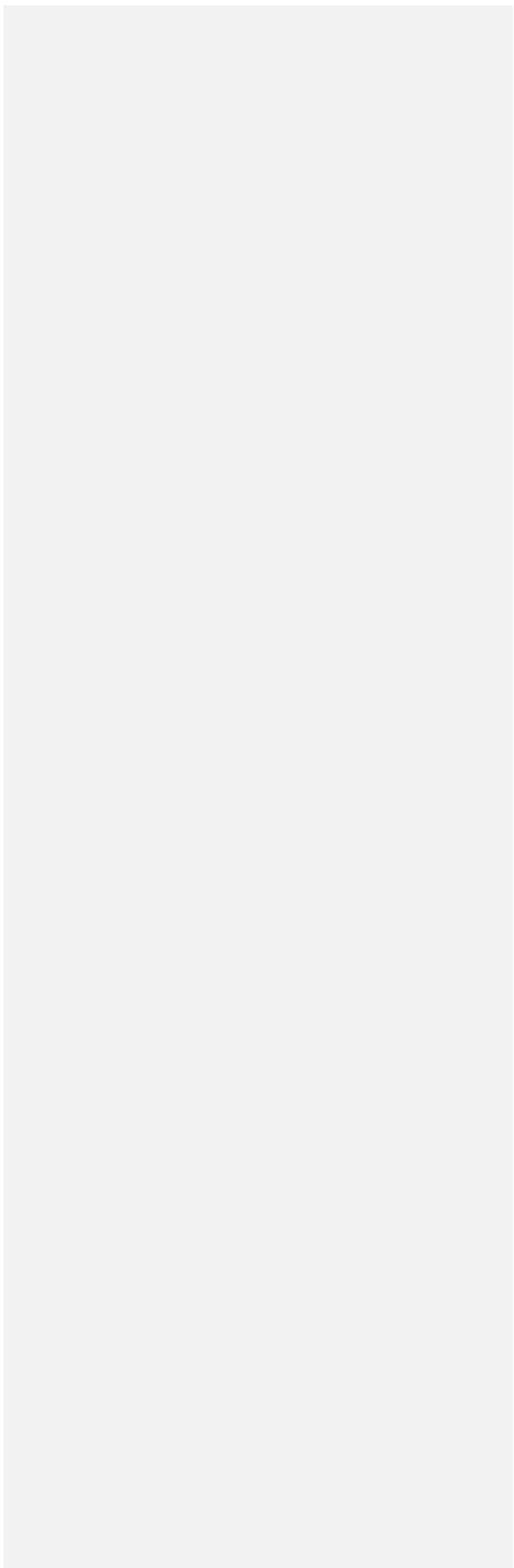


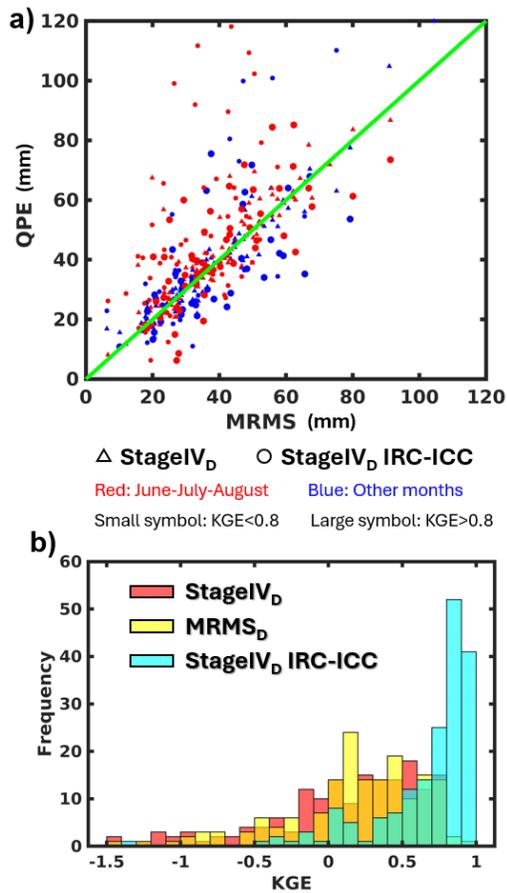
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